USN					



Internal Assessment Test 2 – April 2019

			Int	ternal A	Assessme	nt Test 2	2 – April 2	019				
Sub:	Quantity Sur	Sub Code:	15CV81	1	Branch:	Civil						
Date:	15/4/2019 Duration: 90 min Max Marks: 50 Sem/Sec: VIII – A & B											
			·	Solu	<u>ution</u>	·				MA	RKS	
1	Reduced le 0m to chain 101m and t is 10m and project cost Rs 120/m³ section of t	he road is side slop t for a ler adopting he road	m are give s in upwar pes are 2:1 ngth of 28 g mid sec	en belo rd grad 1 in fill 0m if t	w. The for ient of 1 ling and the cost of	ormation in 400. I 1.5:1 in f filling	level at the Formation cutting. Doi: 180	ne 0m ch width of etermine /m³ and of tudinal a	ainag f the r the r cuttin	ge is road road g is	15]	

100.8

101.9

102.4

102.5

Ans Given Data:

RL of

Ground

Formation level at zero chainage = 101m (with upward gradient of I in 400)

100.2

Formation width (B) = 10m

100.6

100.2

Side slope in cutting = 1.5

Side slope in filling = 2

Cost of filling = $Rs 180/m^3$, Cost of cutting = $Rs 120/m^3$

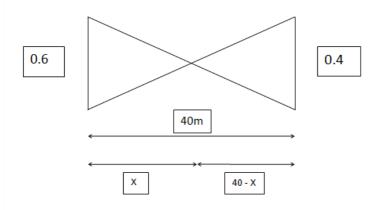
Method: Mid Sectional Area

Calculation for Formation level: given upward gradient of 1 in 400

99.8

For every 400m horizontal distance, the elevation is increased by 1m For every 40m horizontal distance, the elevation is increased by 1/400*40 = 0.1m

Calculation for no cutting and no banking chainage



T 41 1	C	1
Harthwork	tor	raad
Earthwork	$1\mathbf{O}1$	Tuau

		RL of							
	RL of	formati		mean	side	section		quantity	quantity
chainage	ground	on	depth	depth	slope	al area	length	filling	cutting
0	100.6	101	0.4						
40	100.2	101.1	0.9	0.65	2	7.345	40	293.8	
80	99.8	101.2	1.4	1.15	2	14.145	40	565.8	
120	100.2	101.3	1.1	1.25	2	15.625	40	625	
160	100.8	101.4	0.6	0.85	2	9.945	40	397.8	
changes fro	m filling to	cutting							
			0	0.3	2	3.18	24	76.32	
200	101.9	101.5	-0.4	0.2	1.5	2.06	16		32.96
240	102.4	101.6	-0.8	0.6	1.5	6.54	40		261.6
280	102.5	101.7	-0.8	0.8	1.5	8.96	40		358.4
								1958.72	652.96

Cost Abstract

				COSCI	1000100	•
Item						
No.		Particulars	quatity	unit	rate	amount
	L	filling of earthwork	1958.72	m3	180	352569.6
		cutting of				
2	2	earthwork	652.96	m3	120	78355.2

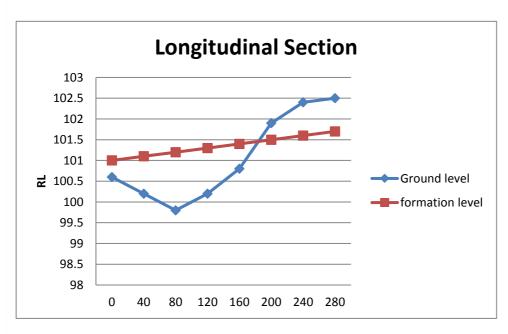
total 430924.8

additional 5% for contingencies and workcharge establishment

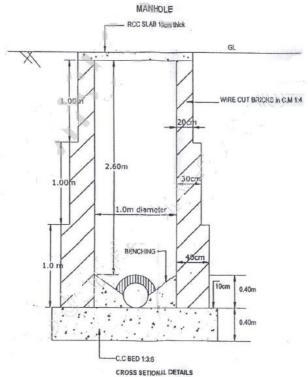
21546.24







- 2 The details of a **circular manhole** are given in fig 1. Find the quantities of the following items.
 - a) Earthwork in excavation
 - b) CC bed 1:3:6
 - c) B.B.M in CM 1:4 for walls
 - d) R.C.C. slab in CC 1:2:4, with 45cm manhole cover
 - e) Plastering in CM 1:3 for side walls

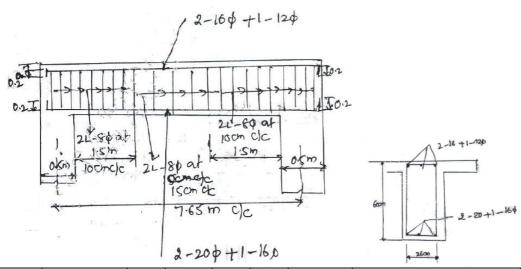


		CROSSS	ETIONAL DETAI	LS		
particulars	No	1	В	ш	0	Remarks
particulars	NO.	L	В	П	Q	
						R=(1+.4+.4+.1+.1)=2.
Earthwork in excavation	1	3.14*1^2		3.5	10.99	0
						H=.4+1+1+1+.1 = 3.5
CC bed 1:3:6						
foundation	1	3.14*1^2		0.4	1.26	
benching	1	3.14*.5^2		0.4	0.314	cylinder
deduct	1	3.14*.35^2		0.2	0.0769	for frustum of cone
					1.4971	
BBM in CM 1:4 for wall						
step 1	1	3.14*1.45	0.45	1	2.04	ave. Dia. = 1.45
Step 2	1	3.14*1.3	0.3	1	1.22	ave. Dia. = 1.3
step 3	1	3.14*1.2	0.2	1	0.753	ave. Dia. = 1.2
					4.013	
RCC slab in CC 1:2:4 with						
45cm manhole cover	1	3.14*.7^2		0.1	0.154	
		3.14*.225^				
deduct man hole cover	1	2		0.1	0.016	
					0.138	
Plastering in CM 1:3 for						
side walls	1	3.14*1		2.6	8.164	H=34
	foundation benching deduct BBM in CM 1:4 for wall step 1 Step 2 step 3 RCC slab in CC 1:2:4 with 45cm manhole cover deduct man hole cover	Earthwork in excavation 1 CC bed 1:3:6 foundation 1 benching 1 deduct 1 BBM in CM 1:4 for wall step 1 1 Step 2 1 step 3 1 RCC slab in CC 1:2:4 with 45cm manhole cover 1 deduct man hole cover 1 Plastering in CM 1:3 for	particulars No. L Earthwork in excavation 1 3.14*1^2 CC bed 1:3:6	particulars No. L B Earthwork in excavation 1 3.14*1^2 CC bed 1:3:6	Earthwork in excavation 1 3.14*1^2 3.5 CC bed 1:3:6 foundation 1 3.14*1^2 0.4 benching 1 3.14*.5^2 0.4 deduct 1 3.14*.35^2 0.2 BBM in CM 1:4 for wall step 1 1 3.14*1.45 0.45 1 Step 2 1 3.14*1.3 0.3 1 step 3 1 3.14*1.2 0.2 1 RCC slab in CC 1:2:4 with 45cm manhole cover 1 3.14*.7^2 0.1 Plastering in CM 1:3 for	particulars No. L B H Q Earthwork in excavation 1 3.14*1^2 3.5 10.99 CC bed 1:3:6 foundation 1 3.14*1^2 0.4 1.26 benching 1 3.14*.5^2 0.4 0.314 deduct 1 3.14*.35^2 0.2 0.0769 BBM in CM 1:4 for wall step 1 1 3.14*1.45 0.45 1 2.04 Step 2 1 3.14*1.3 0.3 1 1.22 step 3 1 3.14*1.2 0.2 1 0.753 RCC slab in CC 1:2:4 with 4 3.14*.225^^ 0.1 0.154 deduct man hole cover 1 3.14*.225^^ 0.1 0.016 Dlastering in CM 1:3 for 0.138

Fig 2 shows the longitudinal section of **RCC beam** of size 300 mm x 600mm. Calculate the quantity of steel required and write bar bending schedule. Take the weight of rod/m as follows

 $\begin{array}{lll} 8mm - 0.4 kg/m, & 12mm - 0.9 \ kg/m, & 20mm - 2.0 \ kg/m, \\ 10mm - 0.6 \ kg/m & 16mm - 1.6 \ kg/m & 25 \ mm - 3.8 \ kg/m \end{array}$

[15]

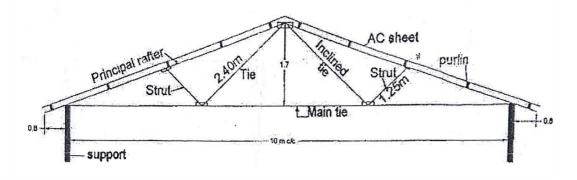


			V	1 .			, 7
item							
no.	particulars	no.	L	В	Н	Q	Remarks
1	RCC work	1	8.15	0.3	0.6	1.467	
2	Steel			density			
	Main Bars						
	of 20mm Ø	2	8.47	2		33.88	L= 8.15 + .2+.20404
	Main Bars						
	of 16mm Ø	1	8.47	1.6		13.552	
	hanger bars						
	of 16mm Ø	2	8.47	1.6		27.104	
	hanger bars						
	of 12mm Ø	1	8.47	0.9		7.623	
	stirrups at		1.78				No= (1.5+.504)/.1 +1 =
	ends	42	4	0.4		29.9712	21
							L= 2(.56+.26)+18*.008
	stirrups in		1.78				
	centre	28	4	0.4		19.9808	No = (8.15-4)/.15 = 28
						132.111	kg

4 Prepare the steel quantity estimate for the **truss** member resting on 40cm wall as shown in the fig 3

Sections of the member

- i) Principal rafter ISA 75X75X8mm @ 8.9 kg/m
- ii) Inclined ties ISA 50x50x6mm @ 4.5 kg/m
- iii) Struts ISA 65x65x6mm @ 5.8 kg/m
- iv) Main Tie ISA 60x60x8 mm @ 7.0 kg/m



[05]

item					unit	total	
no.	particulars	no.	L	В	weight	weight	remarks
	principal						$L=V(6^2+1.7^2) = 6.23: (10+.4+.8+.8)/2=$
1	rafter	2	6		8.9	106.8	6
2	inclined tie	2	2.4		4.5	21.6	
3	strut	2	1.25		5.8	14.5	
4	main tie	1	10.4		7	72.8	L=10+.2+.2 = 10.4
						215.7	kg