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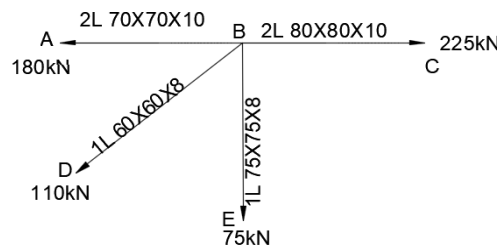
Internal Assessment Test 1 – May. 2021

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|-------|----------------------------|------------|-------------|------------|-------|
| Sub: | Design of steel structures | Sub Code: | 18CV61 | Branch: | Civil |
| Date: | 19/05/2021 | Duration: | 90 min's | Max Marks: | 50 |
| | | Sem / Sec: | 6 A, 6B, 6C | | |

Answer any TWO FULL Questions

Note: Use of IS 800:2007 is permitted and Assume missing data.

| | MARKS | OBE | |
|--|-------|-----|-----|
| | | CO | RBT |
| 1 (a) What are the advantages and disadvantage of using steel structures? | [08] | CO1 | L2 |
| (b) Two flats (Fe 410 grade steel), each 210mm8mm, are to be jointed using 20mm dia, 4.6 grade bolts, to form a lap joint is supposed to transfer a factored load of 250kN. Design the joint and determine suitable pitch for bolts. | [17] | CO1 | L4 |
| 2 (a) Explain the various modes of failure of bolted connection with neat sketch. | [08] | CO1 | L2 |
| (b) Design joint B of a roof truss as shown in Fig. the members are connected with 16mm dia, bolt of grade 4.6 to the gusset plate of 12mm thick. | [17] | CO1 | L4 |



| | | | |
|---|------|-----|----|
| 3 (a) State the limit state design principle. Mention the limit states | [08] | CO1 | L2 |
| (b) Design a bolted connection for lap joint of plate thickness of 10mm and 12mm to carry a service load of 100kN. Use M16 4.6 grade bolt. Give the details with neat sketch. | [17] | CO1 | L4 |

Q1 a) What are the advantages and disadvantages of using Steel Structures?

Advantages

- High Strength
- The high ratio of Strength to weight (the Strength per Unit Weight)
- Excellent ductility and seismic resistance
- With Stand Extensive deformation without failure Even Under high tensile Stress
- Elasticity, Uniformity of material
- Predictability of properties, close to design assumption
- Ease of fabrication and Speed of Erection
- The properties of Steel mostly do not change with time

Disadvantages

- Susceptibility to corrosion
- Maintenance costs/ thin-walled structure
- Loss of Strength at Elevated temperature
- Fireproofing costs
- Susceptibility to buckling
- Fatigue and brittle fracture
- Steel Structure need fire proof treatment which increases the cost
- At the places of Stress concentration in Steel Section Under Section conditions the Steel may losses its durability

4 marks

4Marks

1) (b) Two flats (Fe #10 grade Steel), Each 210mm 8mm, are to be jointed using 20mm dia, 4.6 grade bolts. to form a lap joint is supposed to transfer a factored load of 250kN. Design the joint and determine suitable pitch for bolts

$$f_u = 400 \text{ MPa}$$

$$f_{ub} = 400 \text{ MPa}$$

$$d = 20 \text{ mm}$$

$$d_o = 20 + 2 = 22 \text{ mm}$$

$$A_{nb} = 245 \text{ mm}^2$$

$$\phi_{mb} = 1.25$$

The bolt will be in Single Shear design Shear Strength : V_{dsb}

$$V_{dsb} = \frac{f_{ub}}{\sqrt{3}} (n_n A_{nb} + n_s A_{ns}) / \phi_{mb}$$

$$= \frac{400}{\sqrt{3}} (0.78 \times \frac{\pi}{4} (20)^2) / 1.25$$

$$V_{dsb} = 45.26 \text{ kN}$$

\therefore For this Shear force

$$\text{No of bolts} = \frac{250}{45.26} = 5.52 \approx 6 \text{ no}$$

\therefore Arrange the bolts in three lines as

Now Strength of joint per pitch length on the basis of Shear on bolts

$$= 2 \times 45.26 = 90.52 \text{ kN}$$

Equating this Strength to the net

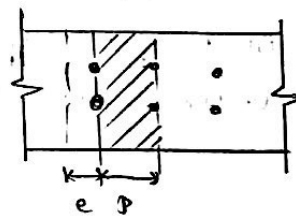
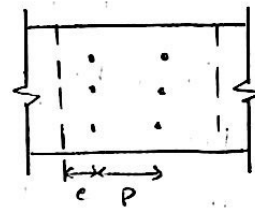
Tensile Strength of plate pitch length

$$T_{dn} = \frac{0.9 f_u (P - d_o) t}{\phi_{mb}}$$

$$90.52 \times 10^3 = \frac{0.9 \times 410 (P - 22) \times 8}{1.25}$$

$$P = 60.32 \text{ mm}$$

$$4 = 65 \text{ mm} \geq 2.5 \times 20 \text{ (ic = 50 mm)}$$



2 Marks

5 Marks

5 Marks

$$\text{Now Edge distance available} = \frac{1}{2} (210 - 2 \times 65) = 40 \text{ mm}$$

$$K_b = \frac{e}{3d_0} ; \frac{p}{3d_0} - 0.25 ; \frac{f_{ub}}{f_u} ; 1$$

$$= \frac{40}{3 \times 22} ; \frac{50}{3 \times 22} - 0.25 ; \frac{400}{410} ; 1$$

$$= 0.6 ; 0.75 ; 0.975 ; 1$$

$$\text{Least Value : } K_b = 0.6$$

\therefore Shear Strength of bolt in bearing

$$V_{dpb} = 2.5 K_b d t f_u / f_{mb}$$

$$= 2.5 \times 0.6 \times 20 \times 8 \times 400 \times 10^{-3} / 1.25$$

$$= 78.72 \text{ kN} > \text{Design Shear Strength} = 45.26 \text{ kN}$$

Hence the design needs no revision.

Q24 Explain the various modes of failure of bolted connection with neat sketch

01. Shear failure of bolt
02. Shear failure of plate
03. ~~Shear~~ Bearing failure of bolt
04. Bearing failure of plate
05. Tensile failure of bolt
06. Tensile failure of plate

01 Shear failure of bolt

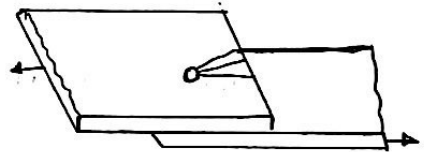
The plates bolted together and subjected to tensile loads may result in the shear of the bolts. The bolts are sheared across their cross sectional areas.



Single shear occurring in a lap joint has been shown in fig

02 Shear failure of plates:

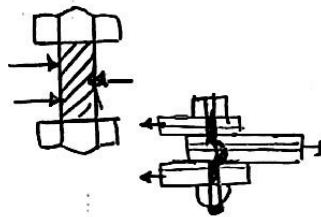
A plate may fail in shear along two lines as shown in fig. This may occur when minimum proper edge distance is not provided.



03 Bearing failure of bolt

The bearing failure of a bolt occurs when the bolt is crushed by the plate as shown.

The bearing, shearing and splitting failure of plates may be avoided by providing adequate edge distance.



04 Bearing failure of plate

The bearing failure of a plate may occur because of insufficient edge distance in the bolted joint. The crushing of plate against the bearing of bolt as shown takes a plate in such failure.

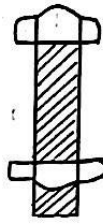


4 Marks

05

Tensile failure of bolt:

The tensile failure of a plate may occur because of insufficient edge distance in the bolted joint the splitting of plate bolt



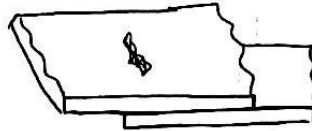
2 Marks

06 Tensile failure of plate:

When plates bolted together are carrying tensile load.

tearing of bolts plates may

When strength of the plate is less than that of the bolts. the tearing failure occurs at net



02) (b) Design joint B of a roof truss as shown in fig.

The members are connected with 16mm dia. bolt of grade 4.6 to the gusset plate of 12mm thick

$$f_u = 410 \text{ MPa}$$

$$f_{ub} = 400 \text{ MPa}$$

$$A_{nb} = 0.78 \times \frac{\pi}{4} \times d^2 = 0.78 \times \frac{\pi}{4} \times (16)^2$$

$$= 157$$

$$D_0 = 16 + 2 = 18 \text{ mm}$$

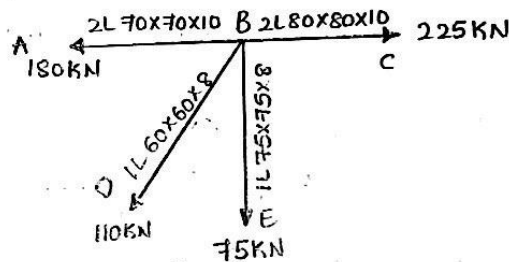
$$\phi_{mb} = 1.25$$

Strength of the bolt in single shear (assume fully threaded bolts) for members BD and BE

$$V_{nsb} = \frac{f_{ub}}{\sqrt{3}} (n_n A_{nb} + n_s A_{sb}) / \phi_{mb}$$

$$= \frac{400}{\sqrt{3}} (1 \times 157 + 0) / 1.25$$

$$V_{nsb} = 29.0 \text{ kN}$$



5 Marks

Strength of the bolt in ϕ double shear (Assume fully threaded)
for members AB and BC

$$= \frac{400}{\sqrt{3}} (2 \times 157 + t) / 1.25 = 58.0 \text{ kN}$$

5 Marks

$$\text{Bolt in Bearing} = V_{spb} = \frac{2.5 \times k_b \times d \times t \times f_u}{f_{mb}}$$

k_b is least of

$$\frac{e}{3d_0} ; \frac{p}{3d_0} - 0.25 ; \frac{f_{yb}}{f_u} ; 1$$

$$\frac{40}{3 \times 18} ; \frac{50}{3 \times 18} - 0.25 ; \frac{400}{410} ; 1$$

$$0.74 ; 0.67 ; 0.97 ; 1$$

3 Marks

Strength of 16mm dia bolt in bearing for different thickness of bolt

$$t = 8 \text{ mm} = (0.25 \times 0.67 \times 16 \times 8 \times 410) / 1.25 = 70.32$$

$$t = 10 \text{ mm} = 0.25 \times 0.67 \times 16 \times 10 \times 410 / 1.25 = 87.9$$

$$t = 12 \text{ mm} = 0.25 \times 0.67 \times 16 \times 12 \times 410 / 1.25 = 105.48$$

• Member AB = 180 kN, 58.0 kN in Shear and 105.48 kN in bearing

$$\text{No of bolts} = \frac{\text{load}}{\text{bolt value}} = \frac{180}{58} = 3.10 = 4 \text{ No}$$

• Member BC = 225 kN, 58.0 kN in Shear and 105.48 kN in bearing

$$\text{No of bolts} = \text{load} / \text{bolt value} = 225 / 58 = 4 \text{ no}$$

4 Marks

• Member of BD = 110 kN, 29.0 kN in Shear and 70.32 kN in bearing

$$\text{No of bolts} = \text{load} / \text{bolt value} = 110 / 29 = 4 \text{ no}$$

• Member BE = 75 kN, 29 kN in Shear and 70.32 kN in bearing

$$\text{No of bolts} = \text{load} / \text{bolt value} = 75 / 29 = 3 \text{ no}$$