

Modified

CBCS SCHEME

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15CV72

Seventh Semester B.E. Degree Examination, Dec.2018/Jan.2019

Design of RCC and Steel Structures

Time: 3 hrs.

Max. Marks: 80

Note: 1. Answer any TWO full questions, choosing one full question from each module.
2. Use of IS-456, IS-800 SP (6) and Steel tables are permitted.

Module-1

- 1 Design a slabtype rectangular combined footing for two columns of size 300mm × 450mm and 300mm × 600mm, subjected to axial loads of 650 kN and 900 kN respectively. The columns are spaced at 3.6 m c/c. The width of the footing is restricted to 1.8 m. Use M20 grade concrete and Fe415 grade steel. Assume SBC of soil = 160 kN/m². (40 Marks)

OR

- 2 Design a Cantilever retaining wall to retain an earth embankment with a horizontal top 3.50 m above ground level. The unit weight of back fill is 18 kN/m³. Angle of internal friction $\phi = 30^\circ$. SBC of soil = 180 kN/m². Take coefficient of friction between soil and concrete = 0.55. Adopt M20 grade concrete and Fe415 grade steel. Depth of foundation = 1.0 m. (40 Marks)

Module-2

- 3 Design a roof truss shown in Fig. Q3 with forces in each member of the truss are given in table Q3. The size of RC column supporting the truss is 300mm × 300mm. Use M20 grade concrete for column. Design the truss using bolt of M16, property class 4.6 for connections and also design anchor bolts. (40 Marks)

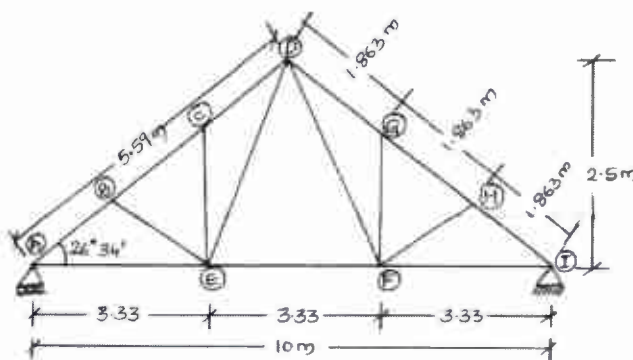


Fig. Q3

Member	Design force in kN	
	Compression	Tension
Top chord member	54.25	-
Bottom chord member	-	48.31
Diagonal member (DF, DE)	14.35	-
Member BE, HF	-	24.50
Member CE, GF	12.40	-

Important Note : 1. On completing your answers, compulsorily draw diagonal cross lines on the remaining blank pages.
2. Any revealing of identification, appeal to evaluator and /or equations written eg. 42+8 = 50, will be treated as malpractice.

OR

4 Design a simply supported crane gantry girder for the following data: The crane is electrically operated. Yield stress of steel is 250 N/mm^2 .

- (i) Span of Crane girder = 20 m
- (ii) Effective span of gantry girder = 7.4 m
- (iii) Capacity of crane = 220 kN.
- (iv) Self weight of Crane girder excluding crab = 200 kN.
- (v) Weight of Crab = 60 kN.
- (vi) Wheel base distance = 3.4 m
- (vii) Minimum hook approach = 1.2 m.
- (viii) Self weight of rail = 300 N/m
- (ix) Height of rail = 75 mm

Gantry girder is to be supported on RCC column bracket of size $300\text{mm} \times 450\text{mm}$. Size of column $300\text{mm} \times 600\text{mm}$. (40 Marks)



Scheme & Solution

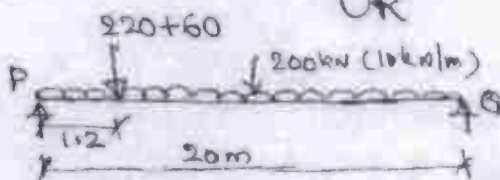
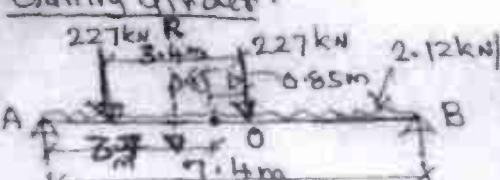
Signature of Scrutinizer

Subject Title: Design of RCC & Steel Structures Subject Code: ISCVT2

Question Number	Solution	Marks Allocated
Q1	<p style="text-align: center;">Module-1</p> <p>Column A = 300mm x 450mm - $P_1 = 650 \text{ kN}$ Column B = 300mm x 600mm - $P_2 = 900 \text{ kN}$ $\text{Total load} = 1550 \text{ kN}$</p> <p>Area of footing = $\frac{1.1 \times 1550}{160} = 10.66 \text{ m}^2$ $B = 1.8 \text{ m}$ (given) $L_{\text{reqd}} = \frac{10.66}{1.8} = 5.92 \text{ m} \approx 6 \text{ m}$</p> <p>Distance of point of application of resultant from column A $\bar{x} = \frac{900 \times 3.6 + 0}{1550} = 2.09 \text{ m}$</p> <p>Extension of footing beyond column A = 0.91m (e/c)</p> <p>Extension of footing beyond centre of col. B = 1.49</p> <p>Soil pressure on footing = $\frac{1550}{6 \times 1.8} = 143.52 \text{ KN/m}^2$ $\text{udl } w = 143.52 \times 1.8 = 258.33 \text{ kN/m}$</p>	<p style="text-align: right;">7 marks</p> <p style="text-align: right;">3 marks</p> <p style="text-align: right;">3 marks</p>

"APPROVED"

Question Number	Solution	Marks Allocated
	<p>Depth required for max. Bm or Depth required for max. one way Shear Assuming percentage of tension steel. and check for one way shear</p> <p>Ast for Maximum +ve Bm Ast for BM under col. A. Ast for BM under col. B. Minimum Ast required.</p> <p>Check for Two way Shear under col. A and col. B - 2x3</p> <p>Ast for transverse bending - Sketches - longitudinal section. -</p>	<p>8 marks</p> <p>8 marks</p> <p>6 marks</p> <p>2 marks</p> <p>3 marks</p> <hr/> <p>40 marks</p>
Q 2.	<p>OR</p> <p>Design of retaining wall.</p> <p>Total height of retaining wall = $3.5 + 1 = 4.5\text{m}$</p> <p>Proportioning of different parts of ret. wall Dimension of base slab, Toeslab, heel slab Thickness of stem & base slab</p> <p>Check for stability of retaining wall</p> <p>Check for Soil pressure at toe & heel</p> <p>Design of stem -</p> <p>Design of heel slab -</p> <p>Design of toe slab - with Sketches (cds)</p>	<p>6 marks</p> <p>8 marks</p> <p>6 marks</p> <p>8 marks</p> <p>6 marks</p> <p>6 marks</p> <hr/> <p>40 marks</p>

Question Number	Solution	Marks Allocated
Q3.	<p style="text-align: center;">Module - 2</p> <p>Design of Top chord members - & Joints</p> <p>Design of bottom chord member - & Joints</p> <p>Design of discontinuous (inner) members subjected to tensile force</p> <p>subjected to compression (including design of joints)</p> <p>Design of Supporting section - Design of baseplate/bearing plate Design of anchor bolt</p>	<p>6 marks</p> <p>8 marks</p> <p>8 marks</p> <p>8 marks</p> <p>5 marks</p> <p>5 marks</p> <hr/> <p>40 marks</p>
Q4.	<p style="text-align: center;">OR</p>  <p> $R_p = \frac{280 \times 18.8}{20} + \frac{200}{2}$ $R_p = 363.2 \text{ kN}$ Each wheel load = $\frac{363.2}{2} = 181.6 \text{ kN}$ + impact load 25% = 45.4 kN Total wheel load = 227 kN Self weight = $\frac{227}{125} = 1.82 \text{ kN/m}$ Weight of rail = 0.30 kN/m Total udl = 2.12 kN/m </p> <p><u>Gantry Girder.</u></p>  <p> $R_B = \frac{227 \times 2 \times (3.7 + 0.85)}{7.4} + \frac{2.12 \times 7.4}{2}$ $R_B = 182.7 \text{ kN}$ Maximum Moment at O, $M_0 = 182.7 \times 2.85 - \frac{2.12 \times 2.85^2}{2}$ $M_0 = 512.085 \text{ kNm}$ </p>	<p>3 marks</p> <p>1 mark</p>
	<p style="text-align: right;">$M_0 = 512.085 \text{ kNm}$</p>	<p>4 marks</p>


Subject Title:

Subject Code:

4

Question Number	Solution	Marks Allocated
	$F_H = \frac{10}{100} \times 280 = 28 \text{ kN} \quad W_H = \frac{28}{2} = 14 \text{ kN}$ $M_y = \frac{520.695}{227} \times 14 = 32.11 \text{ kNm}$ $SF(\text{Max}) = 227 + \frac{227 \times 4}{7.4} + \frac{2.12 \times 7.4}{2} = 357.55 \text{ kN}$ $M_{z,u} = 768.13 \text{ kNm} \quad M_{y,u} = 48.165 \text{ kNm}$ $V_u = 536.33 \text{ kN.}$	2 marks
	Selection of Section, Calculation of \bar{y} , I_{xx} , I_{yy} , Z_e etc. - - - - -	2 marks
	Calculation of EAA, Z_{pz} , Z_{py} - - - - -	3 marks
	Calculation of M_{dz} , M_{dy} & check for stresses.	5 marks
	Check for buckling resistance,	3 marks
	Check for Shear force	5 marks
	check for Longitudinal Stress.	2 marks
	check for deflection.	2 marks
	Design of Connections -	3 marks
	Neat Sketch of C/S at Support - - - - -	3 marks
	* * * * *	2 marks
		<u>40 marks</u>

Approved


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"APPROVED"

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