INTERNAL ASSESSMENT TEST - II DESIGN OF MACHINE ELEMENTS - I

- 1. A shaft of circular section is subjected to a turning moment that fluctuates between 800 kNm and 600 kNm and also bending moment that fluctuates between +500 kNm and -300 kNm. The material selected for the shaft has a stress value of 100 MPa at endurance limit and shear stress value of 120 MPa at the yield point. Determine the diameter of the solid circular shaft taking a value of 2.5 for the factor of safety. Surface factor, size factor and load factor can be taken as 0.9, 0.85 and 1.0 respectively. Shear stress concentration factor is 1.8 and the notch sensitivity is 0.95. (Note: Choose standard diameter in mm)
- 2. A cold drawn steel rod of circular cross section is subjected to a variable bending moment of 565 Nm to 1130 Nm as the axial load varies from 4500N to 13500N. The maximum bending moment occurs at the same instant that the axial load is maximum. Determine the required diameter of the rod (in mm) for a factor of safety =2. Neglect stress concentration effect. Take ultimate stress as 550 MPa., Yield stress as 470MPa and endurance limit as 50% of the ultimate strength and size, load and surface correction coefficients as 0.85, 1 and 0.85 respectively.
 - 3. A weight of 1 kN is dropped from a height of 50 mm at the free end of a cantilever beam of effective length 300 mm. Determine the cross section of the cantilever beam of square cross section (in mm) if the allowable stress in the material of the beam is limited to 80 MPa.

Combined stocks by /& Mt manc = 8004 Nm = 9x108 N-mn m & min = 6004AM = 68108 H mm mbrusc = 50041Mm = 5×108 N-mm mtmin = -300 KNM = -3×108 N-mn T-1= 100pmpa; Zy = 120mpa; Fos=n=27 esz=0.9; esz=0.85; et=1; n=1.8; · Combined . Stock by & oder bereg readion Condider bending load: Bending moment amplitude mber = mbmax - mmin = 5×108 - (-3×103) = 4 ×108 +1-ma mean bending moment mbm = mbmax + mbmin

= 1 ×1081-1-mm Bending Strivs amplitude 5ba = mba = 4x10x

Zb Tyd3 mean bending stress 6 bm = mbm = 1×108

Zb = 1×108

T/32ds = 40,744×167 * consider too sional load: tosque amplitude = m + mesc - mtmn i. e mta = 8 ×108 - 6×108 = 1×108/1-mn meen taque = my max + monin 1. e mem = 8×106+6×10× Shers amplitude = + the Ta = 1 × 108

Zo = 1× 108

The desired in the desired in

mean strey 2m = mon = 7x108 = 25.65x168

Zp = 71/16d3 d2

According to mascinum shor stress theory.

contined mean shear stocks zm = 1/2 JEbm + 422 mm

=1/2 \(\left(\left[0.186\times\16\times\right)^2 +

4 (35.65×108)2

= 3.6×109

combined shear stores amplitude z'' = {\frac{1}{2}} \overline{12} = 1/2 \ (40.48BX103) & + 4 (\$5.89X108) 2 2 2 3 4 4 (\$5.89X108) 2

= 2.4×109

Arohu, according to soderberg's & brught line relation, according to stress

workenbation K-1 Za" + Z"m = 1

Z-Vd = Tyd i.e, N-1 Ta"

6-181.882.852 Ty = 1/n K-E ta"

t t'n

Ty = /n Fatigue Ster concentration factor N-z= 1+9 (Wz-1) = 1+0.02 (1.8-1) =1.41 $\frac{1.76 \times 201 \times 109}{120} + \frac{3.6 \times 109}{120} = \frac{1}{25}$ i. diameter of shaft d = 80.665mm

[d = 600mm]// Given

Combined stress by soderberg/ Goodman relation

(i) rensider the Bending load:

Amplitude bending moment Mba = (1130-565) x 103

282.5 X103 Nmm

mean bending moment Mbm = (1130 + 565) X103

Amplitude Bending & toess 6ba = 32Mba = 287.75×104 Mmm² d3 Mmm²

Mean Bending Stocks. Ebm = 32 Mbm = 863.25 X104 Mmm

Trd3 = 863.25 X104 Mmm

(ii) consider the Axial load: -

hean

Amplitude load, Fa = 4500N

Mean load, Fm = 13500+4500 = 9000111

Amplitude & toess, $6a = \frac{4Fa}{trd^2} = 5.73 \times 10^3 \text{ H/mm}^2$

mean Strass, Em = 4x9000.

11da = 11.45 x 103 da H/mma.

combined mean & box, 6" = 5 pm + 6m = 863.25×104 +11.45×103 Combined Amplitude & bey, 6a' = 6 ba + 6a = 287.75 ×104

d3 + 5.73 × 13

da According to Soderberg Straight line equation (Eq 5.83g-DPB) 5a + 5"m = 1 5-1d = 571 61-d= 6-1 61-1= 0.5×550=275mpr $n-1d=\frac{1}{el-esz-ess}$ Eld = 99.63 M/mm2. 5yd = 6y - 470 = 235 H/mm Sub the above values in equation 5 889 d = 41.23 % 45mm (Standard diameter).

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W= 14N = 1000; h = sommil = 200mn;

6 = somfa; b= d. (= square

Assume E= 206.8×10 4/mm2

Masciman B.m = mb = Wl = 1000 x 300 = 3x 10 5 H mm

Don table, mascinum static deflection y = Well

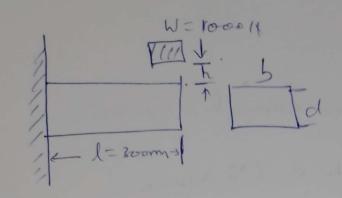
from table 1.3.a $J = \frac{bd^3}{12} = \frac{b^4}{12}$

Sm (0r) y = 1000 × 3003 3 x 206.8 x 103 x (b4/12) = 522.244X103

Bending stress the to static local

6. (m) 6. = Mb I ×5 , where (= d/2 = b/2)

 $= \frac{3 \times 10^{3}}{(6 + 1)^{2}} \cdot \frac{18 \times 10^{5}}{6 \times 10^{3}}$



Impact stress due to bending 6 = 6 [1+ Jitah]

 $1.6 \ , 80 = 18\times10^{5}$ $1+ 1+ 2\times50$ 522.244×10^{3} 54

ie 4.44 44 105 63 = 1+) 1+ 1.9148 × 10 4 64

1. e (4.444×10-563-1).= J1+1.9148×15464

Squaring on both sides

 $1.9753\times10^{-5}b^{3}-8.8889\times10^{-5}b^{3}+1=1+1.9148\times10^{+}$

1.e b3 - 45000 = 96936.756

1.e b3-96936.75b-45000=0

By total and enos method

Lay b= 312m width of beam = depth of Lecemb = 311.58 mm