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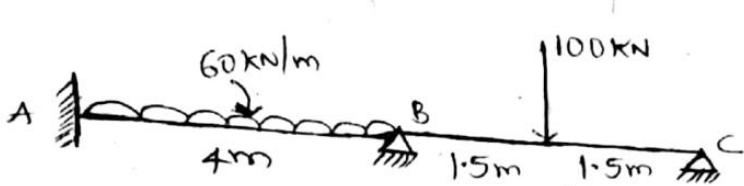
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Internal Assessment Test 4 –Feb. 2022

Sub:	Analysis of Indeterminate Structures				Sub Code:	18CV52	Branch :	Civil	
Date:	1/2/2022	Duration:	90 min's	Max Marks:	50	Sem / Sec:	5A		OBE
<u>Answer TWO FULL Questions</u>							MARK S	CO	RBT
1.	Analyze the Frame show in fig 1. Using Stiffness matrix method. Draw BMD and SFD							[25]	CO1 L3
1.									
2.	Analyze the given frame shown in fig. by Stiffness matrix method.							[25]	CO2 L3
2.	<p style="text-align: center;">fig 2</p>								

Solution

- Analyze the given continuous beam using stiffness matrix method of analysis



Steps

1. Fixed end moments

$$M_{FAB} = -80$$

$$M_{fba} = 80$$

$$M_{fbc} = -37.5$$

$$M_{fcb} = 37.5$$

2. $[\Delta]$, $[P]$, $[P_L]$

a. $[\Delta] = \text{unknown displacement matrix} = \begin{bmatrix} \theta_B \\ \theta_C \end{bmatrix}$

$[P] = \text{moments acting in unit directions} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$

$[P_L] = \text{joint moments} = \begin{bmatrix} M_{FBA} + M_{FBC} \\ M_{FCB} \end{bmatrix} = \begin{bmatrix} 80 - 37.5 \\ 37.5 \end{bmatrix} =$

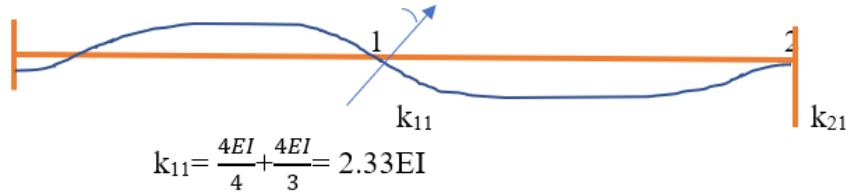
3. Stiffness matrix = to find the θ_B, θ_C

- a. Applying the unit rotation along the 1 co-ordinate directions

$$[\Delta] = [k]^{-1} [P - P_L]$$

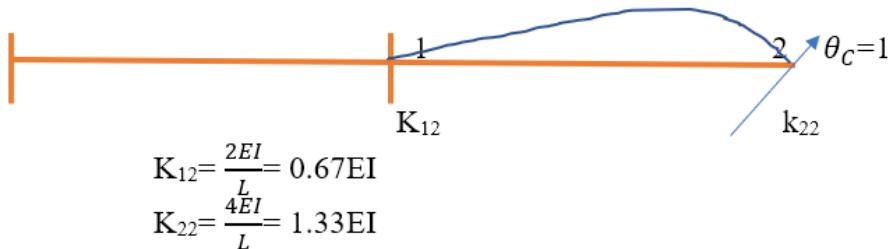
For coordinate 1

$$\theta_B = 1$$



$$k_{21} = \frac{2EI}{L} = 0.67EI$$

applying the unit rotation at co-ordinate 2



$$[k] = \begin{bmatrix} k_{11} & k_{12} \\ k_{21} & k_{22} \end{bmatrix}$$

$$[k] = EI \begin{bmatrix} 2.33 & 0.67 \\ 0.67 & 1.33 \end{bmatrix}$$

$$[k]^{-1} = \frac{1}{EI} \begin{bmatrix} 2.33 & 0.66 \\ 0.67 & 1.33 \end{bmatrix}^{-1}$$

$$[\Delta] = [k]^{-1} [P - P_L]$$

$$\begin{bmatrix} \theta_B \\ \theta_C \end{bmatrix} = \frac{1}{EI} \begin{bmatrix} 2.33 & 0.66 \\ 0.67 & 1.33 \end{bmatrix}^{-1} \begin{bmatrix} 0 - (80 - 37.5) \\ 0 - (37.5) \end{bmatrix} \quad .$$

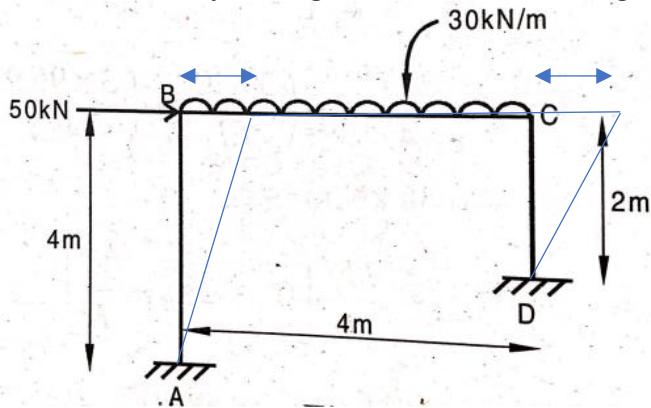
$$\begin{bmatrix} \theta_B \\ \theta_C \end{bmatrix} = \frac{1}{EI} \begin{bmatrix} -11.89 & . \\ -22.36 & . \end{bmatrix}$$

$$\theta_B = -11.89/EI, \theta_C = -22.36/EI$$

4. Slope deflection equation:

$$\begin{aligned} M_{AB} &= M_{FAB} + \frac{2EI}{l}(2\theta_A + \theta_B - \frac{3\delta}{l}) \\ &= -80 + 2EI/4(0 - 11.89/EI - 0) = -85.94 \text{ KN.M} \\ M_{BA} &= M_{FBA} + \frac{2EI}{l}(2\theta_B + \theta_A - \frac{3\delta}{l}) \\ &= 80 + 2EI/4(2/(-11.89/EI)) = 68.11 \text{ kN.m} \\ M_{BC} &= M_{FBC} + \frac{2EI}{l}(2\theta_B + \theta_C - \frac{3\delta}{l}) \\ &= -37.5 + 2EI/3(2 * -11.89/EI - 22.36/EI) = -68.16 \text{ Kn.M} \\ M_{CB} &= M_{FCB} + \frac{2EI}{l}(2\theta_C + \theta_B - \frac{3\delta}{l}) \\ &= 37.5 + 2EI/3(2 * -22.36/EI - 11.89/EI) = -0.05 \text{ kN.m} \end{aligned}$$

Problem 2: Analyze the given frame shown in fig. by Stiffness matrix method.



Unknowns = 3

1. Fixed end moment:

$$M_{fbc} = -40$$

$$M_{fcb} = 40$$

2. $[\Delta], [P], [P_L]$

$$[\Delta] = \text{unknown displacement matrix} = \begin{bmatrix} \delta \\ \theta_B \\ \theta_C \end{bmatrix}$$

$$[P] = \text{External forces} = \begin{bmatrix} 50 \\ 0 \\ 0 \end{bmatrix}$$

$$[P_L] = \text{joint force(moments)} = \begin{bmatrix} 0 \\ M_{FBA} + M_{FBC} \\ M_{FCB} + M_{FCD} \end{bmatrix} = \begin{bmatrix} 0 \\ -40 \\ 40 \end{bmatrix}$$

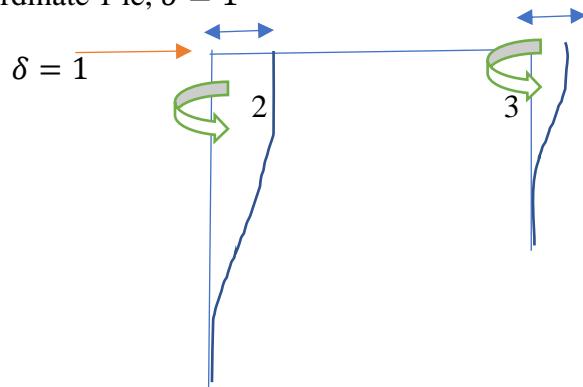
3. Stiffness matrix= to find the $\delta, \theta_B, \theta_C$

Applying the unit rotation along the co-ordinate 1 ie, $\delta = 1$

$$k_{11} = \frac{12EI}{4^3} + \frac{12EI}{2^3} = 1.68EI$$

$$k_{21} = -\frac{6EI}{4^2} = -0.37EI$$

$$k_{31} = -\frac{6EI}{2^2} = -1.5 EI$$

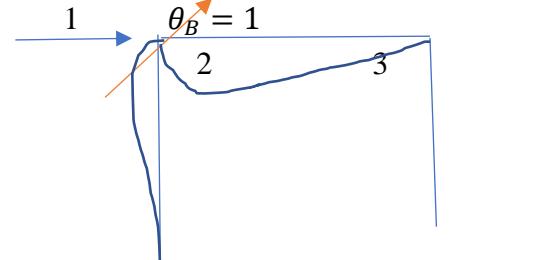


Applying the unit rotation along the co-ordinate 2, ie =

$$k_{12} = -\frac{6EI}{L^2} = -0.37EI$$

$$k_{22} = \frac{4EI}{L} + \frac{4EI}{L} = 2EI$$

$$k_{23} = \frac{2EI}{L} = 0.5EI$$



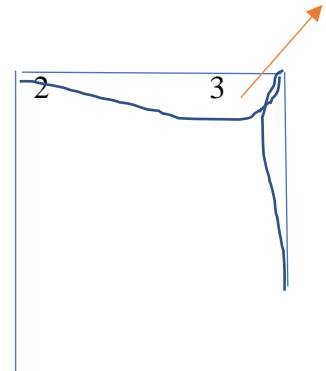
$$\theta_C = 1$$

Applying the unit rotation along the co-ordinate 3

$$K_{13} = -\frac{6EI}{L^2} = -1.5EI$$

$$K_{23} = \frac{2EI}{L} = 0.5EI$$

$$k_{33} = \frac{4EI}{L} + \frac{4EI}{L} = 3EI$$



$$\begin{bmatrix} k_{11} & k_{12} & k_{13} \\ k_{21} & k_{22} & k_{23} \\ k_{31} & k_{32} & k_{33} \end{bmatrix} = [EI] \begin{bmatrix} 1.68 & -0.37 & -1.5 \\ -0.37 & 2 & 0.5 \\ -1.5 & 0.5 & 3 \end{bmatrix}$$

$$[\Delta] = [k]^{-1} [P - P_L]$$

$$\begin{bmatrix} \theta_B \\ \theta_C \\ \theta_D \end{bmatrix} = \frac{1}{EI} \begin{bmatrix} 1.68 & -0.37 & -1.5 \\ -0.37 & 2 & 0.5 \\ -1.5 & 0.5 & 3 \end{bmatrix}^{-1} \begin{bmatrix} 50 \\ 40 \\ -40 \end{bmatrix}$$

$$\delta = 35.68/EI, \theta_B = 26.6/EI, \theta_C = 0.1/EI$$

4. Substitute the above values in the slope deflection equation

$$M_{AB} = M_{FAB} + \frac{2EI}{l} (2\theta_A + \theta_B - \frac{3\delta}{l})$$

$$M_{AB} = -0.3$$

$$M_{BA} = M_{FBA} + \frac{2EI}{l} (\theta_A + 2\theta_B - \frac{3\delta}{l})$$

$$M_{BA} = 13.33$$

For Span BC, l =

$$M_{BC} = M_{FBC} + \frac{2EI}{l} (2\theta_B + \theta_c - \frac{3\delta}{l})$$

$$M_{BC} = -13.33$$

$$M_{CB} = M_{FCB} + \frac{2EI}{l} (\theta_B + 2\theta_c - \frac{3\delta}{l})$$

$$M_{CB} = 53.33$$

For Span CD, l =

$$M_{CD} = M_{FCD} + \frac{2EI}{l} (2\theta_c + \theta_D - \frac{3\delta}{l})$$

$$M_{CD} = -53.33$$

$$M_{DC} = M_{FDC} + \frac{2EI}{l} (2\theta_D + \theta_c - \frac{3\delta}{l})$$

$$M_{DC} = -53.33$$