

Modified

CBGS SCHEME

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18ME32

Third Semester B.E. Degree Examination, Feb./Mar. 2022 Mechanics of Materials

Time: 3 hrs.

Max. Marks: 100

Note: Answer any FIVE full questions, choosing ONE full question from each module.

Module-1

- 1 a. Define i) Poisson's ratio ii) Stress iii) Percentage Reduction in area iv) Toughness. (04 Marks)
- b. Derive the relationship between Modulus of Rigidity and Modulus of elasticity. (06 Marks)
- c. A stepped bar is subjected to an external loading as shown in Fig. Q1(c). Calculate the change in the length of bar. Take $E = 200\text{GPa}$ for steel, $E = 70\text{GPA}$ for Aluminum and $E = 100\text{ GPa}$ for copper. (10 Marks)

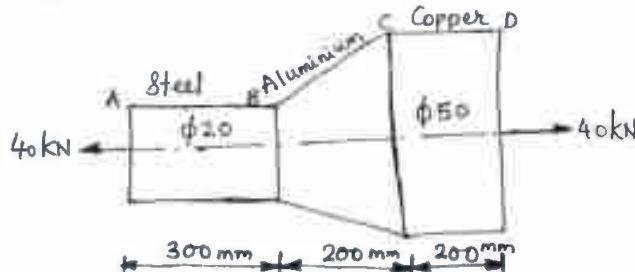


Fig. Q1(c)

OR

- 2 a. Draw Stress – Strain diagram for mild steel subjected to tension and indicate salient points on the diagram. (06 Marks)
- b. A composite section comprises of a steel tube 10cm internal diameter and 12cm external diameter fitted inside a brass tube of 14cm internal diameter and 16cm external diameter. The assembly is subjected to a compressive load of 500kN. Find the load carried by each tube and change in the length of tubes. The length of tube is 150cm. Take $E_S = 200\text{ GPa}$ and $E_B = 100\text{ GPa}$. (08 Marks)
- c. The bronze bar 3m long with 320mm^2 cross sectional area is placed between two rigid walls. At -20°C there is a gap $\Delta = 2.5\text{mm}$ as shown in Fig. Q2(c). Find the magnitude and the type of stress induced in the bar when it is heated to a temperature 60°C . Take $E = 80\text{GPa}$ and $\alpha_B = 18 \times 10^{-6}/^\circ\text{C}$. (06 Marks)

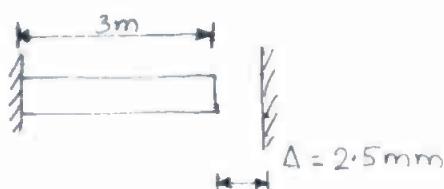


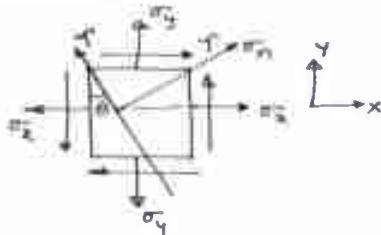
Fig. Q2(c)

Important Note : 1. On completing your answers, compulsorily draw diagonal cross lines on the remaining blank pages.
2. Any revealing of identification, appeal to evaluator and /or equations written eg. $42+8 = 50$, will be treated as malpractice.

Module-2

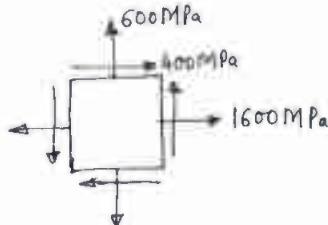
- 3 a. Derive the expression for normal stress and tangential stress on a plane inclined at θ° to the vertical axis in a biaxial stress system with shear stress as shown in Fig.Q3(a). Also find Resultant stress and Angle of Obliquity. (10 Marks)

Fig. Q3(a)



- b. The state of stress at a point in a strained material as shown in Fig. Q3(b). Determine
 i) The principal stresses and principal planes.
 ii) Maximum shear stress and plane on which it is acting. Also find the normal stress on the maximum shear plane.
 iii) Sketch the element aligned with planes of principal stresses and planes of maximum shear. (10 Marks)

Fig. Q3(b)

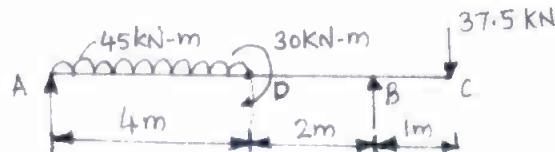
**OR**

- 4 a. A thin cylinder of 75mm internal diameter and 250mm long has 2.5mm thick walls. The cylinder is subjected to an internal pressure of 7MN/m^2 . Determine the change in internal diameter and change in length and change in volume of cylinder. Also compute the Hoop stress and Longitudinal stress and maximum shear stress. Take $E = 200\text{GPa}$ and $\mu = 0.3$. (10 Marks)
- b. A thick cylinder with internal diameter 80mm and external diameter 120mm is subjected to an external pressure of 40kN/m^2 , when the internal pressure is 120kN/m^2 . Calculate the circumferential stress at external and internal surfaces of the cylinder. Plot the variation of circumferential stress and radial pressure on the thickness of the cylinder. (10 Marks)

Module-3

- 5 Draw Shear force and Bending moment diagrams for the beam shown in Fig. Q5. Locate the point of contra flexure if any. (20 Marks)

Fig. Q5

**OR**

- 6 a. A simply supported beam of span 5m has a cross section of $150\text{mm} \times 250\text{mm}$. If the permissible stress is 20N/mm^2 . Find
 i) Maximum intensity of uniformly distributed load it can carry.
 ii) Maximum concentrated load P applied at 2m from one end it can carry. (10 Marks)

- b. The cross section of a beam is a T section (Fig. Q6(b)) $150\text{mm} \times 100\text{mm} \times 15\text{mm}$ with 150mm horizontal. Find the maximum intensity of shear stress and sketch the shear stress distribution across the section if it has to resist a shear force of 90kN. (10 Marks)

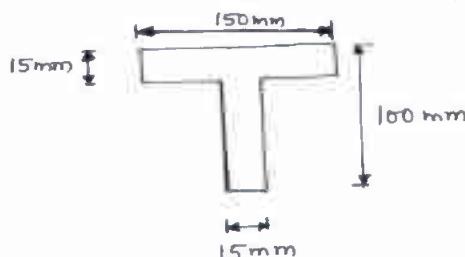


Fig. Q6(b)

Module-4

- 7 a. Derive the torsional equation for a circular shaft with usual notations. State the assumptions made. (10 Marks)
 b. A solid circular shaft is subjected to a bending moment of 10kN-m and a torque of 15kN-m. The yield stress of the material in simple tension is 250MPa and $E = 200\text{GPa}$. If factor of safety is 3. Determine the maximum diameter of the shaft using Maximum Principal Stress theory and Maximum Shear Stress theory. (10 Marks)

OR

- 8 a. Write a note on :
 i) Maximum Principal Stress theory ii) Maximum Shear Stress theory. (08 Marks)
 b. A solid circular shaft is required to transmit 300kW at 120 rpm. The shear stress in the material is not to exceed 80N/mm^2 . Find the diameter required. If the shaft is replaced by a hollow one whose internal diameter is 0.6 times its external diameter. The length material and maximum shear stress being same. Calculate the percentage saving in weight, that could be obtained. (12 Marks)

Module-5

- 9 a. Explain Castiglano's theorem I with its applications and Castiglano's theorem II. (10 Marks)
 b. A hallow cast iron column whose outside diameter is 200mm and thickness of 20mm is 4.5m long and is fixed at both ends. Calculate the safe load by Rankine formula using factor of safety 2.5. Find the ratio of Euler's to Rankine's loads. Take $E = 1 \times 10^5\text{N/mm}^2$ and Rankine constant $= \frac{1}{1600}$ for both ends fixed and $\sigma_c = 550\text{N/mm}^2$. (10 Marks)

OR

- 10 a. Derive an expression for a critical load in a column subjected to compressive load. When one end is fixed and other end is free. (10 Marks)
 b. Calculate the strain energy stored in a bar shown in Fig. Q10(b), subjected to a gradually applied axial load of 80kN. Compare this value with what obtained in uniform bar of same length and having the same volume, when subjected to the same load. $E = 2 \times 10^5\text{N/mm}^2$. (10 Marks)

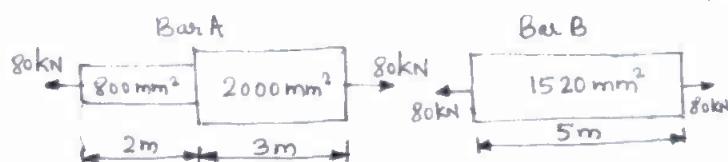


Fig. Q10(b)

Re: Sir, Regarding Modification of Scheme and Solutions

"Dr M S Govinde Gowda" <msggowda1964@gmail.com>
To: boe@vtu.ac.in

May 24, 2022 3:04 PM

Dear Sir,

PFA for Corrected and Approved scheme and solution of 18ME32-Mechanics of Materials for your kind notice and further needful from your end.

Further to mention with regard to Q5 which was claimed as out of syllabus that the most of the BOE members as well the subject experts have expressed that the couple is also a kind of load and also it needs to be taught (even though not mentioned) since this concept is very essential in other subjects like Design-1 and -2, FEM etc. Hence it need not be considered as out of syllabus.

With regards

Dr. M.S.Govinde Gowda

Chairman, BOE, Mechanical Board, VTU

and

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On Wed, May 18, 2022 at 4:21 PM <boe@vtu.ac.in> wrote:

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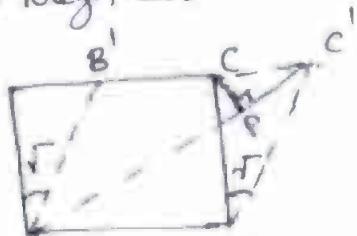
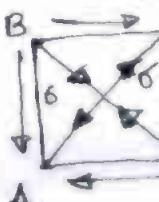
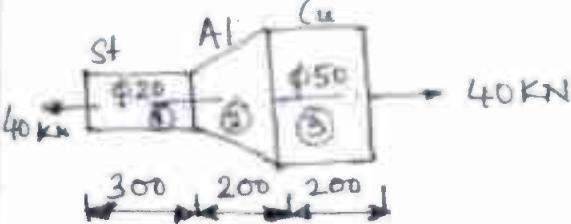

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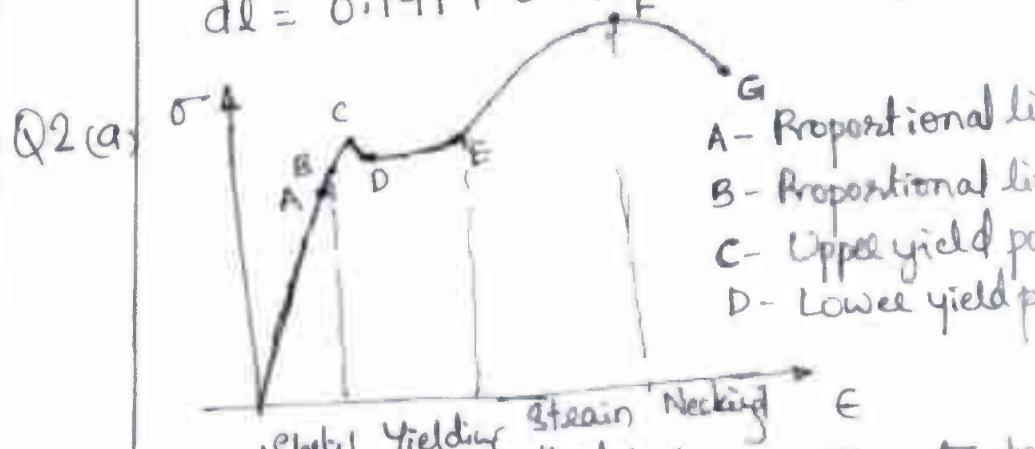
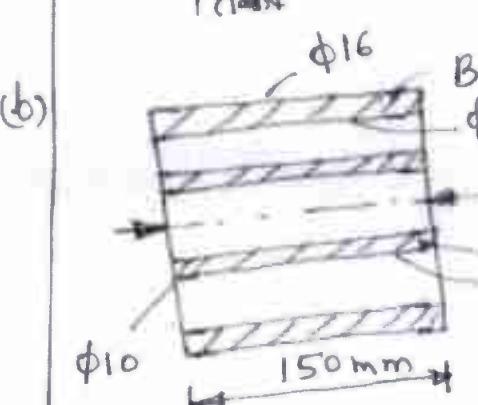
Subject Title : Mechanics of Materials

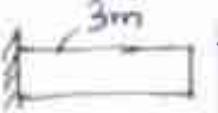
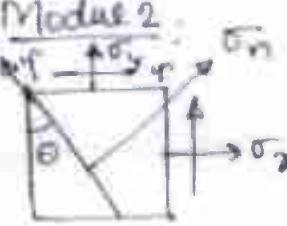
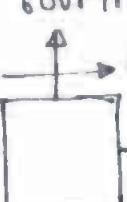
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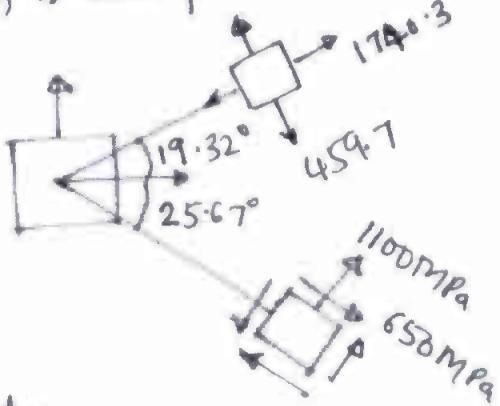
Question Number	Module-1	Solution	Marks Allocated
Q.1(a)			
	(i) Poisson's ratio = $\frac{\text{lateral strain}}{\text{longitudinal strain}}$ within elastic limit	01	01 each
	(ii) Stress = σ_1 (iii) f. Reduction in area = $\frac{A_1 - A_2}{A_1} \times 100$	01	01
	(iv) Toughness	01	01
(b)		Fig - 01	
	$E_{AC} = \frac{PC'}{AC} = \frac{cc'}{\sqrt{2} \cdot \sqrt{2} CD} = \frac{cc'}{2CD} = \frac{\tau}{2} \therefore \tau = \frac{cc'}{CD}$	02	
	$E_{AC} = \frac{\tau}{2G} \therefore \tau = \frac{\tau}{G}$		
	 $E_{AC} = \frac{\sigma_1}{E} + \epsilon \frac{\sigma_2}{E} = \frac{\tau}{E} (1 + \epsilon) \quad (2)$	02	
	From ① & ②		
	$E_{AC} = \frac{\tau}{2G} = \frac{\tau}{E} (1 + \epsilon) \Rightarrow E = 2G (1 + \epsilon)$	01	
(c)		$dl_1 = \frac{Pd_1}{A_1 E_1}$ $= \frac{40 \times 10^3 \times 300}{(\frac{\pi}{4} \times 20^2) 200 \times 10^3}$ $= 0.191 \text{ mm}$	03

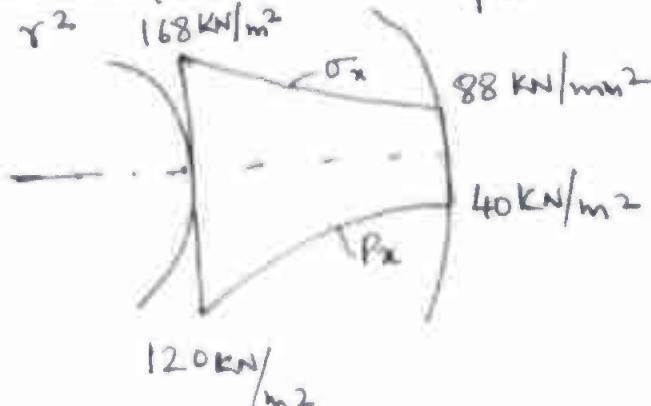
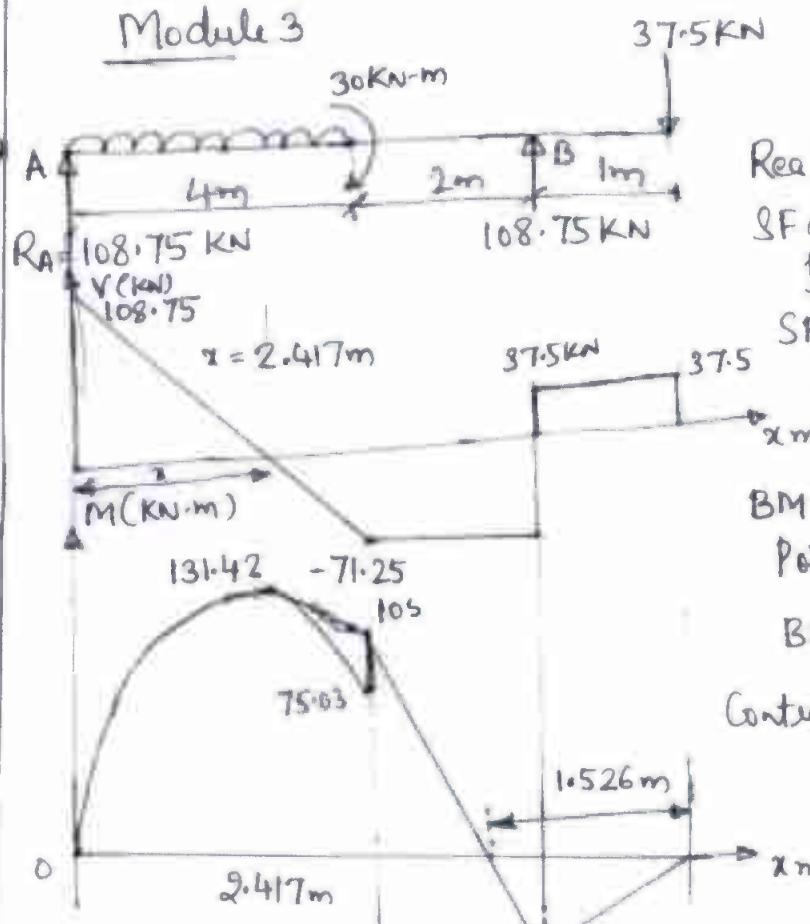
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Question Number	Solution	Marks Allocated
Q1 (C)	$dl_2 = \frac{4Pl_2}{\pi d_1 d_2 E} = \frac{4 \times 40 \times 200}{\pi \times 20 \times 50 \times 70} = 0.146 \text{ mm}$ $dl_3 = \frac{Pl_3}{A_3 E_3} = \frac{40 \times 200}{(\frac{\pi}{4} \times 50^2) \times 100} = 0.041 \text{ mm}$ Total elongation $dl = dl_1 + dl_2 + dl_3$ $dl = 0.191 + 0.146 + 0.041 = 0.378 \text{ mm}$	03
Q2 (a)	 <p>A graph of stress versus strain showing the mechanical behavior of a material. The x-axis is labeled 'Strain' and the y-axis is labeled 'Stress'. The curve passes through points A, B, C, D, E, F, and G. The regions are labeled as follows:</p> <ul style="list-style-type: none"> Elastic Yielding Plastic Strain Hardening Necking F - Ultimate stress G - Breaking Stress <p>Annotations on the graph include:</p> <ul style="list-style-type: none"> A - Proportional limit B - Proportional limit C - Upper yield point D - Lower yield pt 	06
(b)	 <p>Diagram of a composite bar consisting of two materials, Brass and Steel, bonded together. The outer diameter is $\phi 16$, the inner diameter is $\phi 14$, and the total length is 150 mm. The bar is subjected to a total load of 500 kN.</p> <p>Calculation for area of brass:</p> $A_s = \frac{\pi}{4} (d_o^2 - d_i^2)$ $= \frac{\pi}{4} (12^2 - 10^2)$ $= 34.55 \text{ mm}^2$ <p>Calculation for area of steel:</p> $A_B = \frac{\pi}{4} (16^2 - 14^2) = 47.12 \text{ mm}^2$ <p>Equation for total load:</p> $P_s + P_B = P_{\text{Total}} = 500 \text{ kN}$ <p>Equation for elongation due to load:</p> $dl_s = dl_b = \frac{P_s l}{A_s E_s} = \frac{P_B l}{A_B E_B} \Rightarrow \frac{P_s}{34.55 \times 200} = \frac{P_B}{47.12 \times 100}$	01

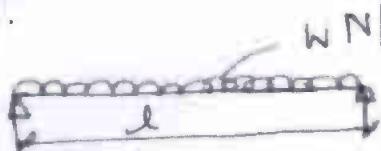
Question Number	Solution	Marks Allocated
	$P_s = 1.466 P_B$ $1.466 P_B + P_B = 500 \text{ kN}$ $P_B = 202.72 \text{ kN} \quad P_s = 297.28 \text{ kN}$	03 01
	$dl = \frac{297.28 \times 150}{34.55 \times 200} = \frac{202.72 \times 150}{47.12 \times 100} = 6.45 \text{ mm}$	02
Q2(c)	 $T_1 = -20^\circ\text{C} \quad T_2 = 60^\circ\text{C}$ $E = 80 \times 10^3 \text{ N/mm}^2$ $\alpha = 2.5 \text{ mm} \quad \alpha_B = 18 \times 10^{-6}/^\circ\text{C}$ $\text{Free expansion} = \alpha L \Delta T$ $\text{Stress induced} = 18 \times 10^{-6} \times 3 \times 10^3 \times 80 = 4.32 \text{ mm}$	02
	$\text{Stress induced } \frac{\sigma \Delta}{E} = \alpha L \Delta T - 2.5 = 4.32 - 2.5 = 1.82 \text{ mm}$	01
	$\sigma = \frac{E \times 1.82}{l} = \frac{80 \times 10^3 \times 1.82}{3 \times 10^3} = 48.53 \text{ N/mm}^2$	03
Q3(a)	 $\sigma_n = \left(\frac{\sigma_x + \sigma_y}{2} \right) + \left(\frac{\sigma_x - \sigma_y}{2} \right) \cos 2\theta + \tau \sin 2\theta$ $\tau = - \left(\frac{\sigma_x - \sigma_y}{2} \right) \sin 2\theta + \tau \cos 2\theta$ $\sigma_R = \sqrt{\sigma_n^2 + \tau^2}, \quad \phi = \tan^{-1} \left(\frac{\tau}{\sigma_n} \right)$	04 04 02
3(b)	 $\sigma_x = 1600 \text{ MPa}$ $\sigma_y = 600 \text{ MPa}$ $\tau = 400 \text{ MPa}$	

Question Number	Solution	Marks Allocated
3(b)	<p>(i) $\sigma_{n_{max}} = \frac{1740.3}{\sqrt{3}} \text{ MPa (T)} \quad \sigma_{n_{min}} = \frac{459.7}{\sqrt{3}} \text{ MPa (T)}$ $1740.3 \text{ MPa} \quad 459.7 \text{ MPa}$</p> $\theta_{np_1} = 19.32^\circ \quad \theta_{np_2} = 90 + 19.32 = 109.32^\circ \quad -04$ <p>(ii) $\sigma_{max} = 640.3 \text{ MPa}$ $\theta_{S_1} = -25.67^\circ \quad \theta_{S_2} = 64.32^\circ \quad -03$ $\sigma_{n \text{ on shear plane}} = 1100 \text{ MPa}$  Module</p>	
Q4(a)	$\sigma_n = \frac{7 \times 75}{2 \times 2.5} = 105 \text{ N/mm}^2 \quad \sigma_e = 52.5 \text{ N/mm}^2 \quad -02$ <p>Max shear stress = 26.25 N/mm^2</p> $\epsilon_d = \frac{7 \times 75}{2 \times 2.5 \times 2 \times 10^5} \left(1 - \frac{0.3}{2}\right) = 4.4625 \times 10^{-4} \quad -03$ $\delta d = 0.033 \text{ mm}$ $\epsilon_e = \frac{7 \times 75}{4 \times 2.5 \times 2 \times 10^5} \left(1 - 2 \times 0.3\right) = 1.05 \times 10^{-4} \quad -03$ $\delta l = 0.02625 \text{ mm}$ $\epsilon_v = 9.975 \times 10^{-4}, \quad dv = 4406.82 \text{ mm}^2 \quad -02$	

Question Number	Solution	Marks Allocated
Q4(b)	$P_x = \frac{B}{r^2} - A$, $120 = \frac{B}{0.04^2} - A$, $40 = \frac{B}{0.06^2} - A$ $A = 24$ $B = 0.2304$ $\sigma_x = \frac{0.2304}{r^2} + 24$ $P_x = \frac{0.2304}{r^2} - 24$ 	- 04 - 02 - 04
Q5	<p><u>Module 3</u></p>  <p>Reactions - 02</p> <p>SF at Salient points - 05</p> <p>SF at $x = 2.417m$ - 01</p> <p>SFD - 02</p> <p>BM at Salient Points - 06</p> <p>BMD - 02</p> <p>Contraflexure - 02</p> <p>Point of contraflexure $x = 1.526m$</p>	- 02 - 05 - 01 - 02 - 06 - 02 - 02

Q No

Q 6(a)

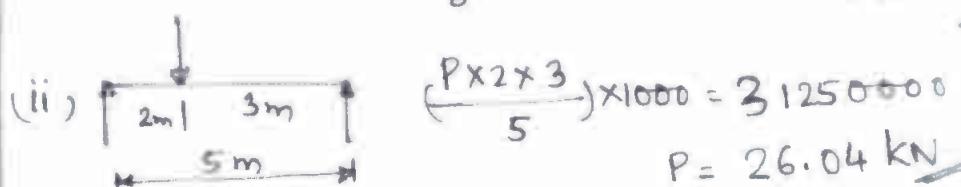


$$I = \frac{bd^3}{12} \quad y_{max} = \frac{d}{2}$$

$$Z = \frac{bd^2}{6} = \frac{150 \times 250^2}{6} = 1562500 \text{ mm}^3$$

$$M = \sigma_{per} Z = 20 \times 1562500 = 31250000 \text{ N-mm}$$

(i) Max BMoment = $\frac{wl^2}{8} = 31250000 \Rightarrow w = 10000 \text{ N/m}$
 $= 10 \text{ kN/m}$



05

05

(b)

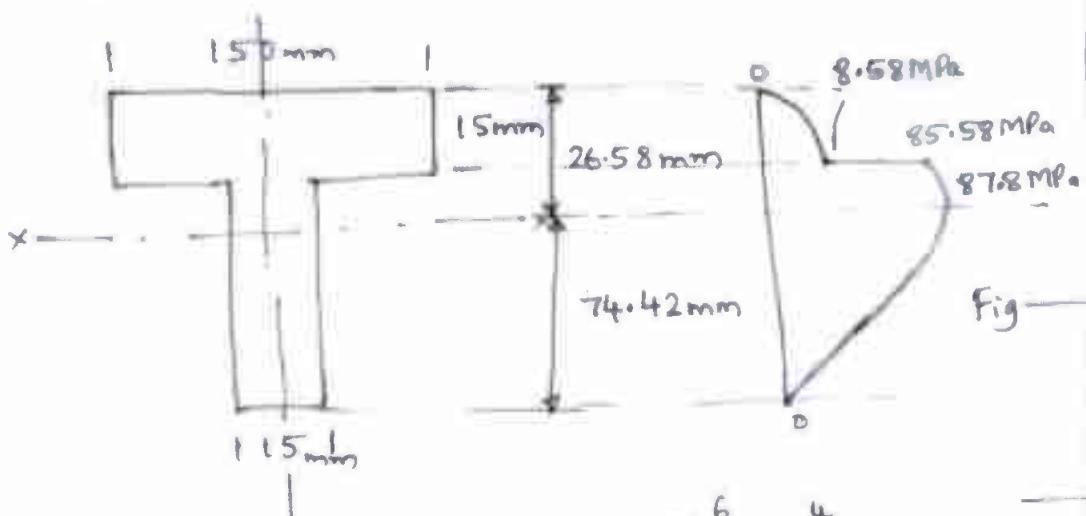


Fig 02

$$\bar{y} = 26.58 \text{ mm} \quad I_{xx} = 2.844 \times 10^6 \text{ mm}^4$$

Shear stress at ~~at~~ salient points

Module 4

Q7(a) Derivation — 08 Assumptions — 02
 $\frac{T}{J} = \frac{T}{R} = \frac{G\theta}{L}$ Derivation — 08
 Assumptions — 02

(b) $M = 10 \text{ kN-m}, T = 15 \text{ kN-m}, \sigma_y t = 250 \text{ N/mm}^2, E = 200 \text{ GPa}, FOS = 3$

$$\sigma_1 = \frac{32M}{\pi d^3} = \frac{32 \times 10 \times 10^6}{\pi d^3} = \frac{101.86 \times 10^6}{d^3} \text{ N/mm}^2$$

$$\tau = \frac{16T}{\pi d^3} = \frac{16 \times 15 \times 10^6}{\pi d^3} = \frac{76.39 \times 10^6}{d^3} \text{ N/mm}^2$$

Max principal stress $\frac{1}{2}(\sigma_1 + \sqrt{\sigma_1^2 + \tau^2}) = \frac{\sigma_y t}{FOS} \Rightarrow d = 119.65 \text{ mm}$

Max shear stress $\sqrt{\sigma_1^2 + 4\tau^2} = \frac{\sigma_y t}{FOS} \Rightarrow d = 103.28 \text{ mm}$

Q8(a) Max principal stress theory — 04
 Max shear stress theory — 04

Q.No. Solution

Marks

8(b) $P = 300 \text{ kW} = 300 \times 10^3 \text{ W}$, $N = 120 \text{ rpm}$, $f = 80 \text{ N/mm}^2$

$$d_i = 0.6 d_o$$

$$T = \frac{60 \times (300 \times 10^3)}{2\pi (120)} = 23.87 \times 10^6 \text{ N mm}$$

$$\frac{T}{J} = \frac{\tau}{R} \Rightarrow \frac{23.87 \times 10^6}{(\frac{\pi}{32} d^4)} = \frac{80}{0.5d} \Rightarrow d = 115 \text{ mm}$$

Hollow shaft

$$J = \frac{\pi}{32} (d_o^4 - 0.6 d_o^4) = 0.0855 d_o^4$$

$$\frac{23.87 \times 10^6}{0.0855 d_o^4} = \frac{80}{0.5 d_o} \Rightarrow d_o = 120.41 \text{ mm}, d_i = 72.25 \text{ mm}$$

$$\% \text{ Saving in wt} = \frac{W_s - W_H}{W_s} \times 100 = 29.86\%$$

Module 5

9(a)

Castigliano's theorem I with applications

Castigliano's theorem II

(b) $I = \frac{\pi \times 200^4}{64} - \frac{\pi \times 160^4}{64} = 46369908 \text{ mm}^4$

$$L_e = \frac{4.5}{2} = 2.25 \text{ m} = 2250 \text{ mm}$$

$$P_E = \frac{\pi^2 EI}{L_e^2} = \frac{\pi^2 \times 1 \times 10^5 \times 46369908}{(2250)^2} = 9040052.3 \text{ N}$$

$$A = \frac{\pi}{4} (200^2 - 160^2) = 11309.73 \text{ mm}^2 \quad K = 64.031 \text{ mm}$$

$$P_R = \frac{\sigma_c A}{1 + \alpha (L_e/K)^2} = \frac{550 \times 11309.73}{1 + \frac{1}{1600} \times \left(\frac{2250}{64.031}\right)^2} = 3510907.3 \text{ N}$$

$$\frac{P_E}{P_R} = \frac{9040052.3}{3510907.3} = 2.575$$

Safe load by Rankine's formula = 1404.36 kN

10(a)

ORDerivation of $P_E = \frac{\pi^2 EI}{(2L)^2}$

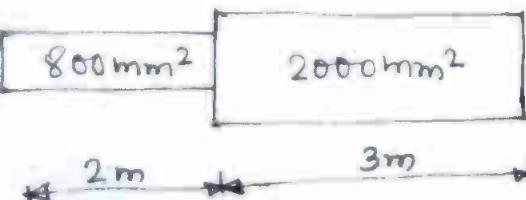
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Q.No

Solution

Marks
allotted

Q10(b)



Beam A

$$U_A = \frac{100^2}{2(2 \times 10^5)} (800 \times 2000) + \frac{40^2}{2(2 \times 10^5)} (2000 \times 3000) = 64 \text{ N-m} \quad (05)$$

$$U_B = \frac{52.63^2}{2(2 \times 10^5)} (1520 \times 5000) = 52.63 \text{ Nm} \quad (04)$$

$$\frac{U_A}{U_B} = \frac{64}{52.63} = 1.216$$

01

Approved by

Dr. M.S. Govinde Gowda
Chairman,
BOE, Mechanical Board, VTU.

END —

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