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**CMRIT**



Internal Assessment Test 3 – January. 2023

Sub:	Basic Geotechnical Engineering				Sub Code:	18CV54	Branch:	Civil Engg
Date:	23.01.2023	Duration:	90 min's	Max Marks:	50	Sem / Sec:	5 A	OBE

Answer any FIVE FULL Questions

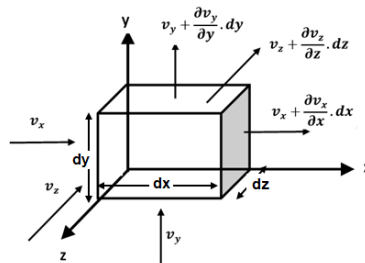
	MARKS	CO	RB T												
1 (a) Define Darcy's law. Derive the Laplace equation for seepage flow.	[10]	CO2	L3												
2 (a) State the characteristics and uses of flow nets.	[10]	CO3	L2												
3 (a) Explain Mohr-Coulomb failure theory of soils.	[10]	CO3	L2												
4 (a) Explain sensitivity and thixotropy of clay.	[10]	CO4	L2												
5 (a) Explain with a neat sketch, determination of pre-consolidation pressure by casagrandes's method and also explain spring analogy theory of consolidation of soil	[10]	CO5	L2												
6 (a) The following data relate to a triaxial compression tests performed on a soil sample: And determine the total & effective stress parameters of soil.	[10]	CO4	L3												
<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>Chamber pressure( <math>\sigma_3</math>)</th> <th>Max. deviator stress (<math>\sigma_1 - \sigma_3</math>)</th> <th>Pore pressure at maximum deviator stress (u)</th> </tr> </thead> <tbody> <tr> <td>80 kN/m<sup>2</sup></td> <td>175kN/m<sup>2</sup></td> <td>45 kN/m<sup>2</sup></td> </tr> <tr> <td>150 kN/m<sup>2</sup></td> <td>240 kN/m<sup>2</sup></td> <td>50 kN/m<sup>2</sup></td> </tr> <tr> <td>210 kN/m<sup>2</sup></td> <td>300 kN/m<sup>2</sup></td> <td>60 kN/m<sup>2</sup></td> </tr> </tbody> </table>	Chamber pressure( $\sigma_3$ )	Max. deviator stress ( $\sigma_1 - \sigma_3$ )	Pore pressure at maximum deviator stress (u)	80 kN/m <sup>2</sup>	175kN/m <sup>2</sup>	45 kN/m <sup>2</sup>	150 kN/m <sup>2</sup>	240 kN/m <sup>2</sup>	50 kN/m <sup>2</sup>	210 kN/m <sup>2</sup>	300 kN/m <sup>2</sup>	60 kN/m <sup>2</sup>	[10]	CO5	L2
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**1. Define Darcy's law. Derive the Laplace equation for seepage flow.**

**Answer:**

**Darcy's Law:** Darcy's law says that the discharge rate  $q$  is proportional to the gradient in hydraulic head and the hydraulic conductivity ( $q = Q/A = -K \cdot dh/dl$ ). 1M

Laplace equation is derived for 3D or 2D flow. The three-dimensional representation of flow is given in Figure below



Consider a controlled volume  $dx \cdot dy \cdot dz$ . Let  $v_x$ ,  $v_y$  and  $v_z$  be the velocity of water with which it enters the controlled volume. Let the velocity with which the water leave the section be

$$v_x + \frac{\partial v_x}{\partial x} \cdot dx$$

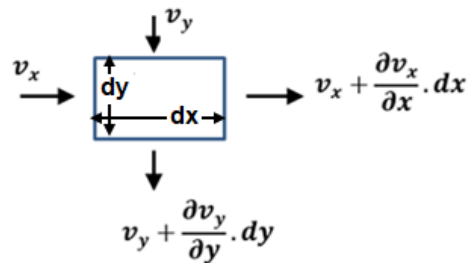
$$v_y + \frac{\partial v_y}{\partial y} \cdot dy$$

$$v_z + \frac{\partial v_z}{\partial z} \cdot dz$$

$$v_x + \frac{\partial v_x}{\partial x} \cdot dx -$$

*means velocity  $\partial v_x$  increases for a small distance of  $\partial x$ . For a distance  $dx$  velocity becomes  $\partial v_x \partial x$ .*

For a 2 Flow the same may be presented as follows in Fig below



According to continuity equation, inflow = outflow. Let the section be of unit width  
Net loss in volume = 0

$$[vx + \partial vx / \partial x \cdot dx - vx] dy + [vy + \partial vy / \partial y \cdot dy - vy] dx = 0 \quad (1)$$

$$\partial vx / \partial x \cdot dx + \partial vy / \partial y \cdot dy = 0 \quad (2)$$

3M

$$\partial vx / \partial x + \partial vy / \partial y = 0 \quad (3)$$

$$v = ki = khL \quad vx = -kx \partial h / \partial x \quad (4)$$

$$vy = -ky \partial h / \partial y \quad (5)$$

$$\text{Substituting (5) and (6) in (3)} \quad (-kx \partial h / \partial x) \partial x + \partial (-ky \partial h / \partial y) \partial y = 0 \quad (7)$$

$$-kx \partial^2 h / \partial x^2 + -ky \partial^2 h / \partial y^2 = 0 \quad (8)$$

3M

For isotropic flow,  $k_x = k_y = k$ . So Equation (8) gets reduced to

$$\partial^2 h / \partial x^2 + \partial^2 h / \partial y^2 = 0 \quad (9)$$

Equation (9) is called as Laplace equation in terms of head.

Velocity potential  $\phi$  is given as  $\phi = -kh \quad (10)$

$$\partial \phi / \partial x = vx = -kx \partial h / \partial x \quad (11) \quad \partial \phi / \partial y = vy = -ky \partial h / \partial y \quad (12)$$

Substituting the values of  $\phi$  in (9), we get Laplace equation in terms of velocity potential

$$\partial^2 \phi / \partial x^2 + \partial^2 \phi / \partial y^2 = 0 \quad (13)$$

**If the soil is anisotropic, then an imaginary section is assumed such that it has a uniform k value given as**

$$k = \sqrt{k_x \cdot k_y}$$

3M

## 2. State the characteristics and uses of flow nets.

**Answer:** A flow net for an isotropic medium is a network of flow lines and equipotential lines intersecting at right angles to each other. The path which a particle of water follows in its course of seepage through a saturated soil mass is called a flow line. Equipotential lines are lines that intersect the flow lines at right angles. At all points along an equipotential line, the water would rise in piezometric tubes to the same elevation known as the piezometric head. 4M

### Properties of flownet

1. Flow and equipotential lines are smooth curves.
2. Flow lines and equipotential lines meet at right angles to each other
3. No two flow lines cross each other.
4. No two flow or equipotential lines start from the same point.
5. Discharge  $\Delta q$  between two flow lines remains constant. Similarly drop in head  $\Delta h$  remains constant between two equipotential drops.
6. The ratio of length and width of each field is constant. Its always considered as squares. 6M

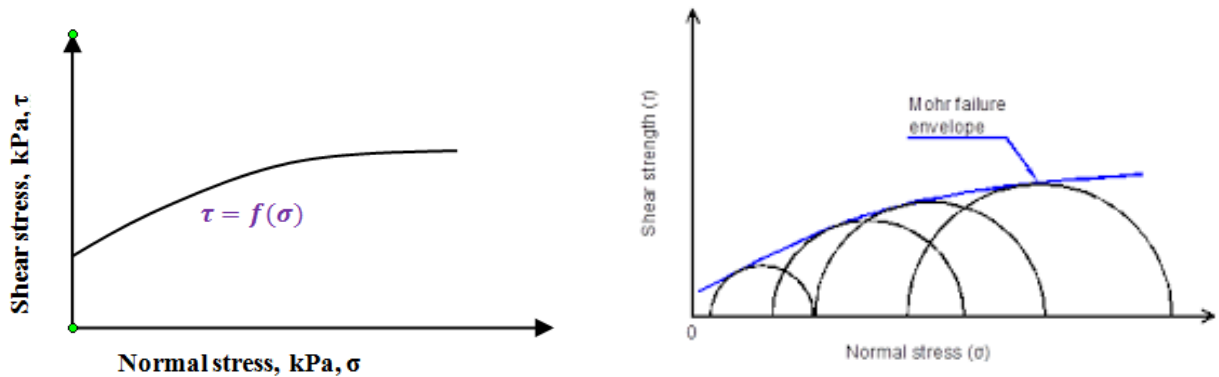
**3. Explain Mohr-Coulomb failure theory of soils.**

**Answer:**

Shear strength of a soil represents the resistance to shear stresses. According to Mohr, failure is caused by a critical combination of normal and shear stresses as represented by equation (1).

Or  $\tau = f(\sigma)$  ---- (1)

Graphically equation (1) will be curved in shape as seen in Fig. At failure, the Mohr failure envelope will be tangential to the Mohr's circle. 2M

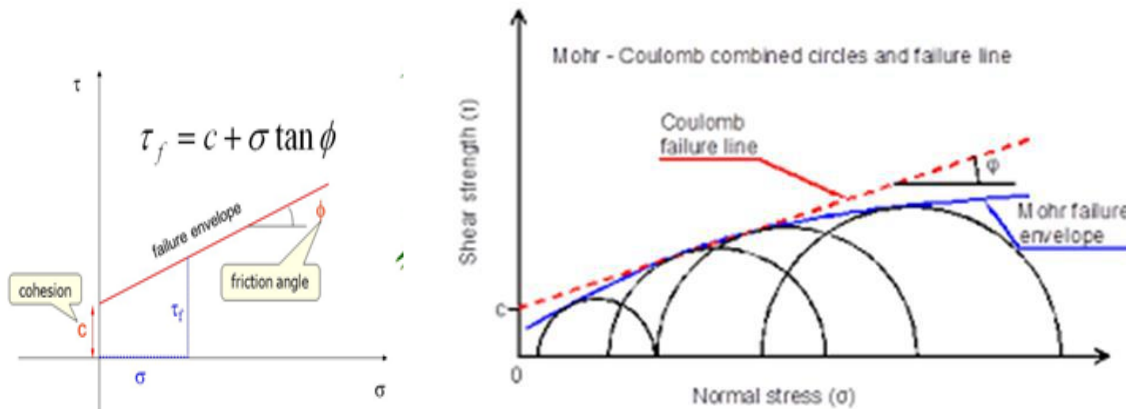


**3M**

Coulomb modified Mohr's theory by stating that shear strength of soil is dependent on two parameters: cohesion between the soil particles and the friction between the particles. Accordingly Equation (1) was modified and the equation for modified failure envelope is given by Equation (2). Mohr's modified failure envelope is given in Fig.5.2

$\tau = c + \sigma \tan \phi$  ---- (2)

**2M**



**3M**

**4. Explain sensitivity and thixotropy of clay.**

**Answer:**

Sensitivity is the measure of loss of strength with remoulding. Sensitivity,  $S_t$  is defined as the ratio of unconfined compressive strength of clay in undisturbed state to unconfined compressive strength of same clay in remoulded state at unaltered water content.

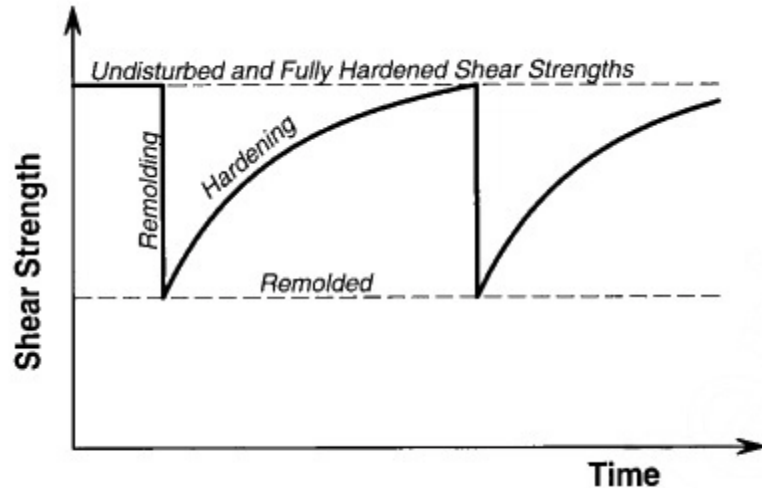
$$S_t = \frac{q_u(\text{undisturbed})}{q_u(\text{remoulded})}$$

<b>S.</b>	<b>No</b>
<b>Classification</b>	
1-4	Normal
4-8	Sensitive
8-15	Extra-sensitive
>15	Quick

5M

The word Thixotropy is derived from two words: thixis meaning touch and tropo meaning to change. Therefore, thixotropy means any change that occurs by touch. If a remolded soil is allowed to stand, without loss of water, it may regain some of its lost strength. In soil engineering, this gain in strength of the true soil with passage of time after it has been remolded is called thixotropy. Thixotropy of soils is of great importance in soil engineering. For example, when a pile is driven into ground, the loss of strength occurs due to disturbance caused. Thixotropy indicates how much shear strength will be regained after the pile has been driven and left in place for some time.

3M



2M

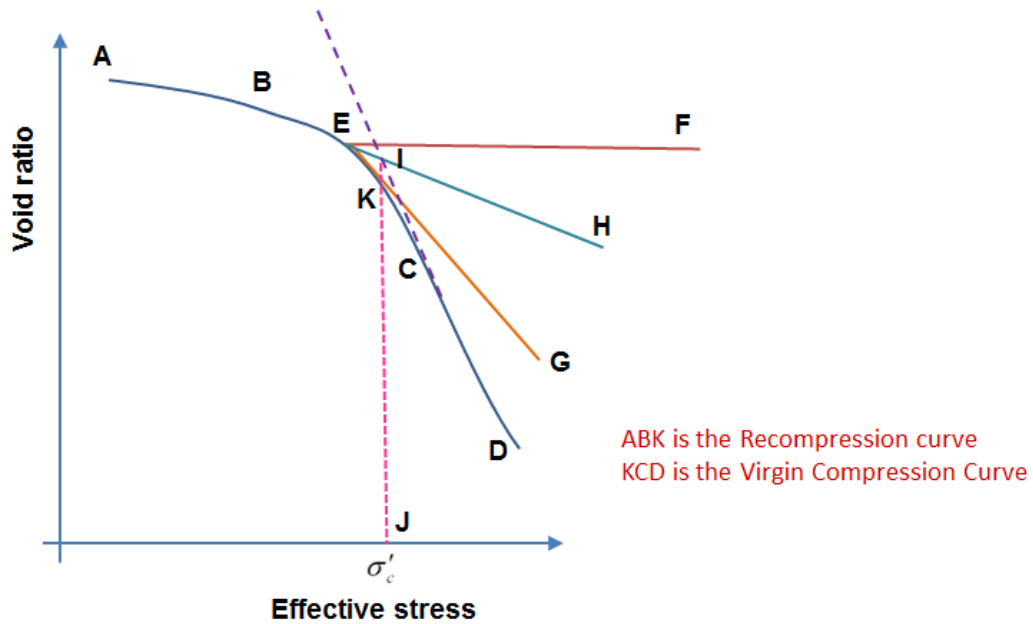
5. Explain with a neat sketch, determination of pre-consolidation pressure by casagrande's method and also explain spring analogy theory of consolidation of soil.

**Answer:**

The maximum pressure to which the soil has been subjected to it, in the past is called as preconsolidation pressure. Casagrande's method for estimating preconsolidation pressure is as explained below:

1. Plot void ratio vs effective stress variation and mark it as ABCD.
2. Choose by eye the point of maximum curvature on the consolidation curve Say E.
3. Draw a horizontal line from this point, line EF.
4. Draw a line tangent to the curve at the point E, line EG.
5. Bisect the angle made from the horizontal line EF and the tangent line EG. Name the bisector as EH.
6. Extend the "straight portion" of the virgin compression curve (high effective stress, low void ratio: almost vertical on the right of the graph) up to the bisector line DG so as to intersect at I.
7. Drop vertical IJ and the abscissa of IJ indicate pre consolidation pressure.
8. Vertical IJ intersect  $e$ - $\log \bar{\sigma}$  curve at K, Curve ABK indicates recompression curve and curve KCD indicate virgin compression curve.

4M



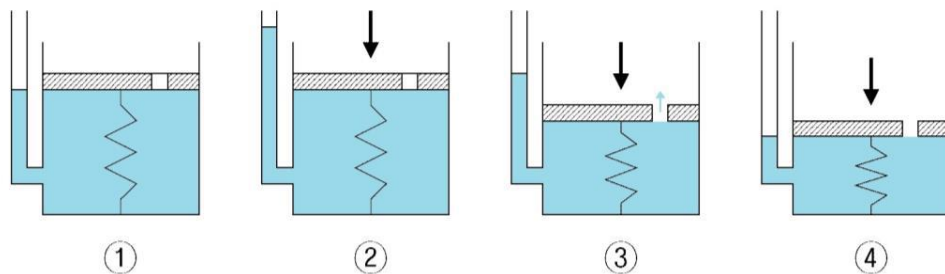
2M

In this system, the spring represents the compressibility or the structure itself of the soil, and the water which fills the container represents the pore water in the soil.

The container is completely filled with water, and the hole is closed. (Fully saturated soil)

- A load of 1 kN is applied onto the cover, while the hole is still unopened. At this stage, only the water resists the applied load. (Development of excessive pore water pressure)
- As soon as the hole is opened, water starts to drain out through the hole and the spring shortens. (Drainage of excessive pore water)
- After some time, the drainage of water no longer occurs. Now, the spring alone resists the applied load. (Full dissipation of excessive pore water pressure. End of consolidation)

3M



1M

6. The following data relate to a triaxial compression tests performed on a soil sample: And determine the total & effective stress parameters of soil.

Chamber pressure( $\sigma_3$ )	Max. deviator stress ( $\sigma_1 - \sigma_3$ )	Pore pressure at maximum deviator stress
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**Department of Civil Engineering**

		(u)
80 kN/m <sup>2</sup>	175kN/m <sup>2</sup>	45 kN/m <sup>2</sup>
150 kN/m <sup>2</sup>	240 kN/m <sup>2</sup>	50 kN/m <sup>2</sup>
210 kN/m <sup>2</sup>	300 kN/m <sup>2</sup>	60 kN/m <sup>2</sup>

Answer:

*Graphical solution:*

a) Total Stress

<i>Test No.</i>	<i>Major Principal Stress (<math>\sigma_1</math>)</i>	<i>Chamber pressure (<math>\sigma_3</math>)</i>
1	255 kN/m <sup>2</sup>	80 kN/m <sup>2</sup>
2	390 kN/m <sup>2</sup>	150 kN/m <sup>2</sup>
3	510 kN/m <sup>2</sup>	210/m <sup>2</sup>

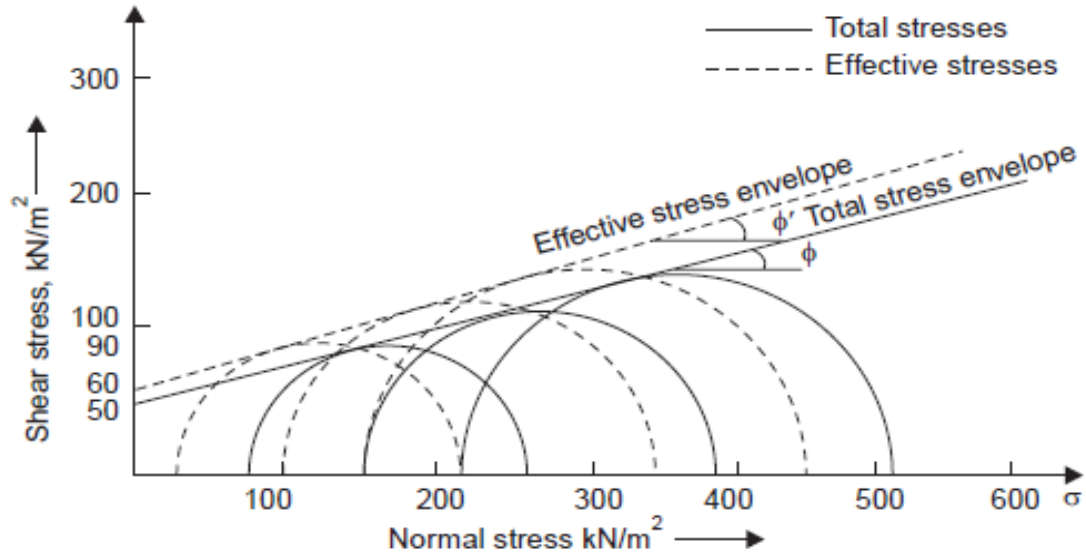
b) *Effective stresses = (Total stress – pore pressure)*

**3M**

<i>Test No.</i>	<i>Major Principal Stress (<math>\sigma_1'</math>)</i>	<i>Chamber pressure (<math>\sigma_3'</math>)</i>
1	210 kN/m <sup>2</sup>	35 kN/m <sup>2</sup>
2	340 kN/m <sup>2</sup>	100 kN/m <sup>2</sup>
3	450 kN/m <sup>2</sup>	150 kN/m <sup>2</sup>



3M



4M

**7. Explain Vane shear test with neat sketch.**

Answer:

If suitable undisturbed or remoulded samples cannot be got for conducting triaxial or unconfined compression tests, the shear strength is determined by a device called the Shear Vane. The vane shear test may also be conducted in the laboratory. The shear vane usually consists of four steel plates welded orthogonally to a steel rod, as shown in Fig below

2M

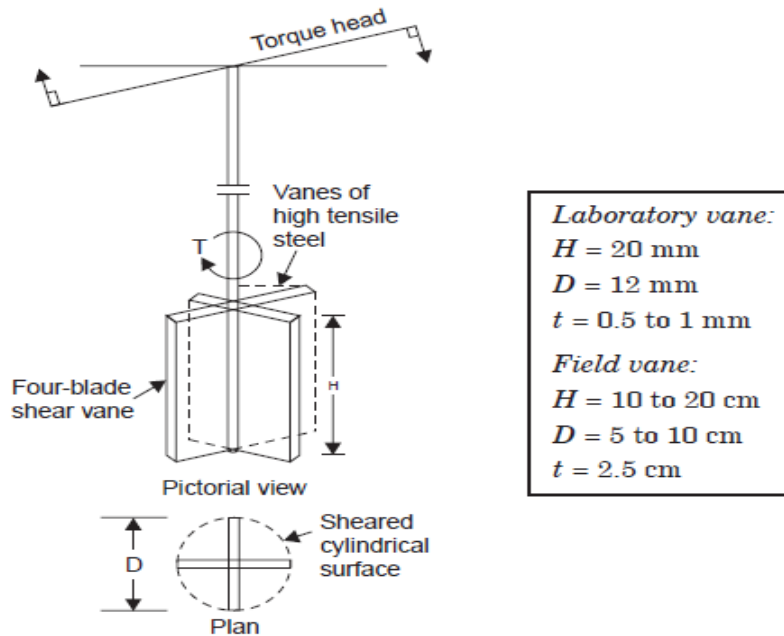


Fig. 5.10. Laboratory shear vane

2M

**Working:**

The applied torque is measured by a calibrated torsion spring, the angle of twist being read on a special gauge. A uniform rotation of about  $1^\circ$  per minute is used. The vane is forced into the soil specimen or into the undisturbed soil at the bottom of a bore-hole in a gentle manner and torque is applied. The torque is computed by multiplying the angle of twist by the spring constant.

$$T = \int_0^{\theta} \pi c u \cdot [D^2 H / 2 + D^3 / 6]$$

$$c u = \int_0^{\theta} T [D^2 H / 2 + D^3 / 6] \quad 3M$$

**Derivation of the formula**

Here the shear strength of the soil is constant for the cylindrical sheared surface as well as for the top and bottom of the specimen.  $T = T_1 + T_2$

For the top and bottom surface,

$$T_1 = 2 \int_0^{D/2} c u \cdot 2\pi r \cdot dr$$

$$T_1 = 2 c u \cdot 2\pi [r^3 / 3]_0^{D/2}$$

$$T_1 = c u [D^3 / 6]$$

For cylindrical surface,

$$T_2 = c u \cdot 2\pi D / 2 \cdot H D / 2$$

$$T_2 = c u \cdot \pi D^2 H / 2 \quad 3M$$